## Preface

Materials that are either manufactured or occur in nature and used both in industry and in our daily lives (metals, rocks, wood, soil, suspensions, and biological tissue) are very seldom homogeneous, and have complicated internal structures. Although the combination of two or more constituents to produce materials with controlled distinguish properties has been exploited since at least ancient civilization, modern composite materials were developed only a few decades ago and have found intensive application in contemporary life and in all branches of industry. Establishment of a link between the structure and properties in order to understand which kind of structure provides the necessary properties is an objective of "micromechanics," which exploits information about microtopology and properties of constituents of the heterogeneous medium for development of mathematical models predicting the macroproperties. The problem of micromechanical modeling of the mechanical properties of engineering materials is today a crucial part of the design process, and sample testing is usually performed only during the final stage for validation of the "virtual" design. An accuracy of the classical "trial and error" testing method of the new materials and constructions is no longer be affordable in modern industry and science.

Owing to wide applications of composite materials, their modeling has been developed very intensively over recent decades, as reflected in the numerous papers and books only partially presented in the reference section of this book. A variety of materials and approaches appearing in apparently different contexts and among different scientific disciplines (solid mechanics, geophysics, solid-state physics, hydromechanics, biomechanics, chemical technology, etc.) do not allow the opportunity to investigate adequately the whole field in a single book. In light of this, I was challenged with a natural question as to why it was necessary to write another book and what is the difference between this book and the ones published earlier. In parallel with this book, there are a few fundamental books combining readily applicable results useful for material scientists with a significant contribution to progress in theoretical research itself. However, a fundamental difference of this book is a systematic analysis of statistical distributions of local microfields rather than only effective properties based on the average field concentrator factors inside the phases.

## VIII Preface

The uniqueness of this book consists of the development and expressive representation of statistical methods quantitatively describing random structures which are most adopted for the subsequent evaluation of a wide variety of macroscopic transport, electromagnetic, and elastic properties of heterogeneous media. The popular methods in micromechanics are essentially one-particle ones that are invariant with respect to statistical second and higher order quantities examining the association of one particle relative to other particles. This book expressively reflects the explosive progress of modern micromechanics resulting from the development of image analyses and computer-simulation methods on one hand and improved materials processing on the other hand, since processing controls the prescribed microstructure. With the appearance of new experimental techniques, it is now possible to study the microtopology of disordered materials much more deeply to understand their properties. Modern techniques are also available to design materials with morphological properties that are suitable for planned applications. This progress in micromechanics is based on methods allowing for statistical mechanics of a multiparticle system considering $n$-point correlation functions and direct multiparticle interaction of inclusions, and the book presents a universally rigorous scheme for both analyses of microstructures and prediction of macroscopic properties which leaves room for correction of their individual elements if improved methods are utilized for the analysis of these individual elements. The book successfully combines advanced numerical methods for the analysis of a finite number of interacting inhomogeneities in either the bounded or unbounded domain with analytical methods.

It should be mentioned that there are two coupled classes of micromechanical problems for which averaging is critical. Averaging is usually suitable for predicting effective elastic properties. However, failure and elastoplastic deformations will depend on specific details of the local stress fields when fluctuation is important. In the framework of computational micromechanics in such a case, one must check the specific observable stress fields for many large system realizations of the microstructure with the use of an extremal statistical technique. A more effective approach used in analytical mechanics is the estimation of the statistical moments of the stress field at the interface between the matrix and inhomogeneities. The inherent characteristics of this interface are critical to understanding the failure mechanisms usually localized at the interface. In the framework of a unique scheme of the proposed multiparticle effective field method (MEFM), we attempted to analyze the wide class of statical and dynamical, local and nonlocal, linear and nonlinear micromechanical problems of composite materials with deterministic (periodic and nonperiodic), random (statistically homogeneous and inhomogeneous, so-called graded) and mixed (periodic structures with random imperfections) structures in bounded and unbounded domains, containing coated or uncoated inclusions of any shape and orientation and subjected to coupled or uncoupled, homogeneous or inhomogeneous external fields of different physical natures.

I do not pretend to cover the whole field of micromechanics in this book (restricted by my interests); there are many other topics in micromechanics, of much industrial and scientific importance, that are either treated schematically or only mentioned. In particular, the homogenization theory of periodic
structures, the geometrically nonlinear problems, flow in porous media, viscoelasticity problems, and cross-property relations are not considered at all in this book while stochastic geometry, variation methods, propagation of waves in composites, and multiscale discrete modeling are not treated in depth they deserve and sometimes were only mentioned. Interested readers are referred to the references cited in the appropriate sections to achieve a deeper understanding of these topics. This book finalizes my research in micromechanics that began with the papers published in 1986. It was written piece by piece at different times and in different countries facing new challenging problems in micromechanics. The book is suitable as a reference for researchers in different disciplines (applied mathematicians; physicists; geophysicists; material scientists; and electrical, chemical, civil, and mechanical engineers) working in micromechanics of heterogeneous media and providing a rigorous interdisciplinary treatment through experimental investigation. The book is also appropriate as a textbook for an advanced graduate course.

I gratefully acknowledge the financial support for 30 years of research in micromechanics of heterogeneous materials by the National Research Council, and the Air Force Office of Scientific Research (AFOSR) of the United States, Austrian Society for the Promotion of Scientific Research, Max-Planck-Society of Germany, as well as by the Ministry of Higher Education, National Academy of Science, and the Ministry of Machine Construction of USSR and Russia. This book would not have been possible without this support. The excellent cooperation with the team at Springer is gratefully acknowledged; I owe special thanks to E. Tham, C. Simpson, and C. Womersly for their careful and patient editing of this book. Also, thank you Elsevier for allowing permission to use Figs. $3.1-3.3,4.12,4.13,5.1-5.4,8.1,9.5,9.6,11.1-11.8,12.2,13.5-13.11,14.1-14.4$, $15.1-15.5,15.8-15.11$, Tables 11.1-11.5 originally published in [133], [134], [136], [137], [146], [168], [179], [182], [184], [190], [191], [633]; Birkhäuser Verlag AG for permission to use Figs. 4.10, 4.11, 10.1-10.6, 15.7, originally published in [145], [152]; SAGE Publication for permission to use Figs. 4.4, 4.6-4.10, 12.4-12.6, originally published in [166], [167]; ASME for allowing permission to use Figs. 12.11, 15.7, 16.10-16.14, originally published in [138], [187]; Begell House Inc. for permission to use Figs. 4.1-4.3, 12.3, 12.12, originally published in [144], [153], [195]; Springer Science and Business Media for allowing permission to use Figs. 12.1, $12.7-12.10,15.6,16.6,16.7,18.7-18.19$ originally published in [131], [155], [185].

My scientific interests go back to my student days in Moscow State University, where I had the good fortune of being nurtured by brilliant teachers and scientists such as, e.g., V. I. Arnold, G. I. Barenblat, A. A. Ilyushin, A. N. Kolmogorov, S. P. Novikov, A. N. Shiryaev, and my first research adviser O. A. Oleinik. In later years I met most of the key players in the development of the subject, many of whom have had a profound influence on my own work. I would like to acknowledge G. J. Dvorak, J. Fish, W. Kreher, E. Kröner, V. I. Kushch, A. M. Lipanov, N. J. Pagano, V. Z. Parton, F. G. Rammerstorfer, T. D. Shermergor, S. Torquato, and G. J. Weng for the helpful and inspiring exchanges of ideas, fantastic collaboration, stimulating discussions, encouraging remarks, and thoughtful criticism I have received over the years. At the risk of forgetting to include some names, I thank the staff of Air Force Research

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I am aware of the fact that the book may contain controversial statements, too personal or one-sided arguments, inaccuracies, and typographical errors. Any the remarks and comments regarding the book will be fully appreciated (buryach@woh.rr.com).

## Foundations of Solid Mechanics

A major problem in continuum mechanics consists of predicting the deformations and internal forces arising in a material body subjected to a given set of external fields such as the forces of pure mechanical nature, temperature and electromagnetic fields. This Chapter summaries some of important definitions, relations, and methods commonly employed for the analysis of reformable media although, of course, many aspects of mechanical behavior are left unaccounted for in this approach and referring the unsatisfied reader to monographs in which a level of higher generality is adopted. However, it is hoped that sufficient details are given so that the reader can understand remainder Chapters without constant reference to other books.

As is customary in continuum mechanics studies, material properties and fields are expressed in tensor form in this book. The general technique of tensor analysis is given, e.g. in [99], [1028], [1068] presenting the tensor theory not only as an autonomous mathematical discipline, but also as a preparation for theories of continuum mechanics. The special applications of tensors are described in the books [722], [1114]. The books [21], [364] provide an introduction to the theories of linear elasticity and nonlinear elasticity. More comprehensive treatments of nonlinear and anisotropic elasticity can be found, e.g., in the books [398], [410], [995], [1015], [1082], [832], [722], and [1098]. The coupling effects of mechanical, temperature and electromagnetic fields are considered in [980], [747], [861]. The general reviews of phenomenological plasticity with an extensive list of references are given in [527], [737], [711].

### 2.1 Elements of Tensor Analysis

This Section provides a remainder of basic notions of tensors of a certain rank which components transforming in a particular way under the transformation of coordinates $\left(\mathbf{e}_{1}, \mathbf{e}_{2}, \mathbf{e}_{3}\right) \rightarrow\left(\mathbf{e}_{1}^{\prime}, \mathbf{e}_{2}^{\prime}, \mathbf{e}_{3}^{\prime}\right)$.

We concern with the rectangular Cartesian coordinate systems and require that the orthonormalized basis is right-handedness $\mathbf{e}_{1} \cdot\left(\mathbf{e}_{2} \times \mathbf{e}_{3}\right)=1$ and $\mathbf{e}_{i} \cdot \mathbf{e}_{i}=$ $\delta_{i j}$, where a scalar multiplication $\mathbf{e}_{i} \cdot \mathbf{e}_{j}$ denotes the projection of the $i$ th basic vector on the $j$ th one (or vice-versa), and

$$
\delta_{i j}= \begin{cases}1 & \text { if } i=j  \tag{2.1}\\ 0 & \text { otherwise }\end{cases}
$$

is the Kronecker delta. The vector (cross) product of two basic vectors $\mathbf{e}_{i} \times \mathbf{e}_{j}$ can be defined by the permutation tensor $\epsilon$ (summation over repeated Latin indices is implied)

$$
\mathbf{e}_{i} \times \mathbf{e}_{j}=\epsilon_{i j k} \mathbf{e}_{k}, \epsilon_{i j k}= \begin{cases}+1 & \text { if } i j k \text { is an even permutation of 123 }  \tag{2.2}\\ -1 & \text { if } i j k \text { is an odd permutation of } 123 \\ 0 & \text { if any two indices are identical. }\end{cases}
$$

The vector product of the vectors $\mathbf{a}, \mathbf{b}$ and the triple scalar product of the vectors $\mathbf{a}, \mathbf{b}, \mathbf{c}$ can consequently be written as

$$
\mathbf{a} \times \mathbf{b}=\epsilon_{i j k} a_{i} b_{j} \mathbf{e}_{k}, \quad(\mathbf{a} \times \mathbf{b}) \cdot \mathbf{c}=\epsilon_{i j k} a_{i} b_{j} c_{k}=\left|\begin{array}{ccc}
a_{1} & b_{1} & c_{1}  \tag{2.3}\\
a_{2} & b_{2} & c_{2} \\
a_{3} & b_{3} & c_{3}
\end{array}\right| .
$$

The expansion of another orthonormalized basis $\mathbf{e}^{\prime}\left(\mathbf{e}_{i}^{\prime} \cdot \mathbf{e}_{j}^{\prime}=\delta_{i j}\right)$ with the same origin of coordinates into the old one $\mathbf{e}_{i^{\prime}}^{\prime}=g_{i^{\prime} j} \mathbf{e}_{j}$ is defined by coefficients $g_{i^{\prime} j}$ called a cosine matrix, because each element $g_{i^{\prime} j}$ is the cosine of the angle between two corresponding axes

$$
\begin{equation*}
g_{i^{\prime} j}=\mathbf{e}_{i}^{\prime} \cdot \mathbf{e}_{j}=\cos \alpha_{i^{\prime} j} \tag{2.4}
\end{equation*}
$$

The most frequently used methods of parametric representation of orthogonal transformation of Cartesian coordinates $\mathbf{e}^{\prime}$ and $\mathbf{e}$ are presented in Appendix A.1. An arbitrary vector $\mathbf{x}$ is completely defined by its magnitude (modulus) and direction in the space

$$
\begin{equation*}
\mathbf{x}=\sum x_{i} \mathbf{e}_{i}=x_{i} \mathbf{e}_{i}, x_{i}=\mathbf{x} \cdot \mathbf{e}_{i}=|\mathbf{x}| \cos \left(\mathbf{x}, \mathbf{e}_{i}\right) \tag{2.5}
\end{equation*}
$$

The orthogonal matrix $\mathbf{g}=\left\|g_{i j}\right\|,(i, j=1, \ldots, 3)$ called the rotation matrix links the coordinates $\mathbf{x}^{\prime}$ of an arbitrary point in the reference orthonormalized frame $\mathbf{e}^{\prime}\left(\mathbf{e}_{i}^{\prime} \cdot \mathbf{e}_{j}^{\prime}=\delta_{i j}\right)$ with the coordinates $\mathbf{x}$ in the crystal orthonormalized coordinate system $\mathbf{e}\left(\mathbf{e}_{i} \cdot \mathbf{e}_{j}=\delta_{i j}\right)$ with the same origin of coordinates (see for details Appendix A.1)

$$
\begin{equation*}
\mathrm{x}^{\prime}=\mathrm{gx} \tag{2.6}
\end{equation*}
$$

The definitions of tensors of order 0 (scalar) ${ }^{0} \mathbf{T}\left(\mathbf{e}_{1}, \mathbf{e}_{2}, \mathbf{e}_{3}\right) \equiv{ }^{0} \mathbf{T}^{\prime}\left(\mathbf{e}_{1}^{\prime}, \mathbf{e}_{2}^{\prime}, \mathbf{e}_{3}^{\prime}\right)$ and 1 [contravariant vector (2.6)] can be easily generalized to contravariant tensors of order $n$

$$
\begin{equation*}
{ }^{n} \mathbf{T}={ }^{n} T_{i_{1} i_{2} \ldots i_{n}} \mathbf{e}_{i_{1}} \otimes \ldots \mathbf{e}_{i n}={ }^{n} T_{i_{1} i_{2} \ldots i_{n}}^{\prime} \mathbf{e}_{i_{1}}^{\prime} \otimes \ldots \mathbf{e}_{i n}^{\prime} \tag{2.7}
\end{equation*}
$$

where the $3^{n}$ tensor components in the prime and unprime coordinates are linked by the following transformation

$$
\begin{equation*}
{ }^{n} T_{j_{1} j_{2} \ldots j_{n}}^{\prime}=g_{j_{1} i_{1}} \ldots g_{j_{n} i_{n}}{ }^{n} T_{i_{1} i_{2} \ldots i_{n}} . \tag{2.8}
\end{equation*}
$$

A tensor ${ }^{n+m} \mathbf{T}$ of rank $n+m$, contravariant of rank $n$ and covariant of rank $m$, is an object with the induced component transformation

$$
\begin{equation*}
{ }^{n+m} T_{j_{1}^{\prime} j_{2}^{\prime} \ldots j_{m}^{\prime}}^{\prime j_{1} j_{2} \ldots j_{n}}=g_{j_{1} i_{1}} \ldots g_{j_{n} i_{n}} g_{i_{1}^{\prime} j_{1}^{\prime}} \ldots g_{i_{m}^{\prime} j_{m}^{\prime}}{ }^{n+m} T_{i_{1}^{\prime} i_{2} \ldots i_{m}^{\prime}}^{i_{1} i_{2} \ldots i_{n}} \tag{2.9}
\end{equation*}
$$

It is possible to distinguish four operations with the tensors. The first one joins the summation and multiplication over the number of the tensors $T_{i_{1}, \ldots, i_{n}}$ and $S_{i_{1}, \ldots, i_{n}}$ of the same order:

$$
\begin{equation*}
\lambda T_{i_{1}, \ldots, i_{n}}+\beta S_{i_{1}, \ldots, i_{n}}=R_{i_{1}, \ldots, i_{n}} \Leftrightarrow \lambda \mathbf{T}+\beta \mathbf{S}=\mathbf{R} \tag{2.10}
\end{equation*}
$$

where $\lambda$ and $\beta$ are scalars. The tensor (or diadic) production of two tensors of orders $n$ and $m$ is a tensor of order $n+m$, defined as

$$
\begin{equation*}
\mathbf{T} \otimes \mathbf{S}=\left(T_{i_{1}, \ldots, i_{n}} S_{j_{1}, \ldots, j_{m}}\right) \mathbf{e}_{i_{1}} \ldots \mathbf{e}_{i_{n}} \mathbf{e}_{j_{1}} \ldots \mathbf{e}_{j_{m}} \tag{2.11}
\end{equation*}
$$

In so doing, in general $\mathbf{T}$ and $\mathbf{S}$ are not commutative, $\mathbf{T} \otimes \mathbf{S} \neq \mathbf{S} \otimes \mathbf{T}$. The third operation called convolution (or contraction) is the action with one tensor reducing his order on two in the framework of the Einstein summation convention, for example

$$
\begin{equation*}
T_{i_{1}, \ldots, i_{n}, k, k, j_{1}, \ldots, j_{m}}=\mathbf{S}_{i_{1}, \ldots, i_{n}, j_{1}, \ldots, j_{m}} \tag{2.12}
\end{equation*}
$$

The fourth operation produces the index permutation leads to a new tensor of the same order but with different index order. For example, one can generates only one new tensor (called transposed one) of the second order: $\mathbf{S}=\mathbf{T}^{\top} \Leftrightarrow S_{i j}=T_{j i}$. In so doing, a second order tensor $\mathbf{T}$ is called symmetric if $\mathbf{T}^{\top}=\mathbf{T}$, and it is called antisymmetric if $\mathbf{T}=-\mathbf{T}^{\top}$.

By straightforward check we are able to ascertain that the new objects formed by mentioned operations are really the tensors. These operations can be arranged in the different combinations. For example, the combination of the diadic production and convolution denoting by a dot leads to the following tensors

$$
\begin{align*}
\mathbf{R} & =\mathbf{T} \cdot \mathbf{S} \Leftrightarrow R_{i j}=T_{i k} S_{k j}  \tag{2.13}\\
\lambda & =\mathbf{R} \cdot \mathbf{T} \Leftrightarrow \lambda=R_{i j} T_{j i}  \tag{2.14}\\
\lambda & =\mathbf{R}: \mathbf{T} \Leftrightarrow \lambda=R_{i j} T_{i j} \tag{2.15}
\end{align*}
$$

where multiple dots are appropriate for complex constructions.
In particular, the simplest tensors ${ }^{2} \mathbf{T}$ of order 2 are the unit tensor $\boldsymbol{\delta}(1.1)$ and the rotation tensor $\mathbf{g}$ (2.4). They hold the property of orthogonality: ${ }^{2} \mathbf{T} \cdot{ }^{2} \mathbf{T}=\boldsymbol{\delta}$ and isotropy: ${ }^{2} \mathbf{T}^{\prime}={ }^{2} \mathbf{T}$. The tensor $\boldsymbol{\delta}$ transform a vector a into itself while $\mathbf{g}$ transforms the unit base vector $\mathbf{e}_{i}$ to the unit vector $\mathbf{e}_{i}^{\prime}$ of the primed coordinate system: $\mathbf{e}_{i}^{\prime}=\mathbf{g} \cdot \mathbf{e}_{i}$. The simplest tensor of order 4 is the unit tensor $\mathbf{I}$ with the components $I_{i j k l}=\frac{1}{2}\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right)$ can be decomposed into the bulk and deviatoric parts

$$
\begin{equation*}
\mathbf{I}=\mathbf{N}_{1}+\mathbf{N}_{2}, \quad \mathbf{N}_{1} \equiv \frac{1}{3} \boldsymbol{\delta} \otimes \boldsymbol{\delta}, \quad \mathbf{N}_{2}=\mathbf{I}-\mathbf{N}_{1} \tag{2.16}
\end{equation*}
$$

which have the property of orthogonality

$$
\begin{equation*}
\mathbf{N}_{1}: \mathbf{N}_{1}=\mathbf{N}_{1}, \mathbf{N}_{2}: \mathbf{N}_{2}=\mathbf{N}_{2}, \mathbf{N}_{1}: \mathbf{N}_{2}=\mathbf{0} \tag{2.17}
\end{equation*}
$$

Differentiation denoting by the del operator $\partial_{i} \equiv \partial / \partial x_{i}$ of a tensor with the suitably smooth components forms another tensors

$$
\begin{align*}
\nabla \cdot \mathbf{T} & \equiv \operatorname{Div} \mathbf{T}=\left(\partial_{j} T_{j i_{1}, \ldots, i_{n-1}}\right)(\mathbf{x}) \mathbf{e}_{i_{1}} \otimes \mathbf{e}_{i_{2}} \ldots \mathbf{e}_{i_{n-1}}  \tag{2.18}\\
\nabla \otimes \mathbf{T} & \equiv \operatorname{Grad} \mathbf{T}=\left(\partial_{i_{1}} T_{i_{2}, \ldots, i_{n+1}}\right)(\mathbf{x}) \mathbf{e}_{i_{1}} \otimes \mathbf{e}_{i_{2}} \ldots \mathbf{e}_{i_{n+1}}  \tag{2.19}\\
\nabla \times \mathbf{T} & \equiv \operatorname{Curl} \mathbf{T}=\left[\epsilon_{i_{1} j k}\left(\partial_{j} T_{k, i_{2}, \ldots, i_{n}}\right)\right](\mathbf{x}) \mathbf{e}_{i_{1}} \otimes \mathbf{e}_{i_{2}} \ldots \mathbf{e}_{i_{n+1}} \tag{2.20}
\end{align*}
$$

which are called divergence, gradient, and curl of the tensor field $\mathbf{T}={ }^{n} \mathbf{T}$ and have the orders $n-1, n+1$, and $n$, respectively. We will also use the other differential operators of the first-and second-order tensors:

$$
\begin{align*}
\text { Def } \mathbf{a} & \equiv \frac{1}{2}(\nabla \otimes \mathbf{a}+\mathbf{a} \otimes \nabla)=\frac{1}{2}\left(a_{i, j}+a_{j, i}\right) \mathbf{e}_{i} \otimes \mathbf{e}_{j}  \tag{2.21}\\
\text { Inc } \mathbf{b} & \equiv \epsilon_{i j k} \epsilon_{l m n} b_{j n, k m} \mathbf{e}_{i} \otimes \mathbf{e}_{j} \tag{2.22}
\end{align*}
$$

called the operator of deformation, and incompatibility, respectively.
Analogously to differentiation, the integration of a tensor field $\mathbf{T}$ on both the domain $w$ and their boundary $\partial w$ can be defined in terms of the integration of its components. The Gauss theorem relates the action of the del operator on the tensor field $\mathbf{T}$ in domain $\mathcal{E}$ to its flux across the boundary $\partial \mathcal{E}$, under suitable smoothness of both the tensor $\mathbf{T}$ and the domain boundary $\partial w$ with the unit vector of an outward normal $\mathbf{n}(\mathbf{s}) \perp \partial \mathcal{E}(\mathbf{s} \in \partial \mathcal{E})$ :

$$
\begin{equation*}
\int_{\mathcal{E}} \nabla * \mathbf{T} d \mathbf{x}=\int_{\partial \mathcal{E}} \mathbf{n} * \mathbf{T} d \mathbf{s} \tag{2.23}
\end{equation*}
$$

where $*$ can stand for ". ", $\otimes$, or $\times$. Let $\partial \mathcal{E}^{\prime}$ be a portion of an oriented surface with unit outward normal $\mathbf{n}$ and with the surface closed edge $C$. Then for the tensor field $\mathbf{T}$ continuously differentiable in $\partial \mathcal{E}^{\prime}$ and continuous in $C$, the generalized Stokes theorem can be presented in the form

$$
\begin{equation*}
\int_{\partial \mathcal{E}^{\prime}}(\mathbf{n} \times \nabla) * \mathbf{T} d \mathbf{s}=\oint_{C} \mathbf{T} * d \mathbf{l} \tag{2.24}
\end{equation*}
$$

Equation (2.23) is also generalized for the volume $\mathcal{E}$ and surface $\partial \mathcal{E}$, excluding the points of a discontinuity surface $\tau$ with unit normal $\mathbf{n}$ which may be sweeping the body. In a similar manner, the surface $\partial \mathcal{E}^{\prime}$ with edge $C$ in Eq. (2.24) is accompanied by a discontinuity line $\gamma$ that may be sweeping the surface. Thus (see e.g., [316]),

$$
\begin{align*}
\int_{\mathcal{E} \backslash \tau} \nabla \cdot \mathbf{T} d \mathbf{x} & =\int_{\partial \mathcal{E} \backslash \tau} \mathbf{T} \cdot \mathbf{n} d \mathbf{s}-\int_{\tau}[\mathbf{T}] \cdot \mathbf{n} d \mathbf{s},  \tag{2.25}\\
\int_{\partial \mathcal{E}^{\prime} \backslash \gamma}(\mathbf{n} \times \nabla) \cdot \mathbf{T} d \mathbf{s} & =\oint_{C \backslash \gamma} \mathbf{T} \cdot d \mathbf{l}-\oint_{\gamma}[\mathbf{T}] d \mathbf{l}, \tag{2.26}
\end{align*}
$$

where the brackets [•] denote the jump of its enclosure across the discontinuity surface $\tau$ or the discontinuity line $\gamma ; \mathcal{E} \backslash \tau, \partial \mathcal{E} \backslash \tau$, and $\partial \mathcal{E}^{\prime} \backslash \gamma, C \backslash \gamma$ exclude points of $\tau$ and $\gamma$, respectively.

### 2.2 The Theory of Strains and Stresses

We consider a body occupying the regions $\mathcal{E}_{0}$ and $\mathcal{E}$, with boundaries $\mathcal{E}_{0}$ and $\partial \mathcal{E}$, in some fixed reference configuration and in the current one, respectively. Suppose
an arbitrary material point $P^{0} \in \mathcal{E}_{0}$ at time $t=0$ has the spatial coordinates $\mathbf{x}$ in the orthonormalized coordinate system $\mathbf{e}_{i}(i=1,2,3)$. The point $P^{0}$ moves during the time $t$ of deformations to the point $P \in \mathcal{E}$ with coordinates $\boldsymbol{\xi}(t)$ in the same coordinate system. The relations between the coordinates $\boldsymbol{\xi}$ and $\mathbf{x}$ are defined by the smooth one-to-one invertible functions:

$$
\begin{equation*}
\boldsymbol{\xi}=\boldsymbol{\xi}(\mathbf{x}, t),\left(\xi_{i}=\xi_{i}\left(x_{1}, x_{2}, x_{3}, t\right)\right) \text { and } \mathbf{x}=\mathbf{x}(\boldsymbol{\xi}, t),\left(x_{i}=x_{i}\left(\xi_{1}, \xi_{2}, \xi_{3}, t\right)\right) \tag{2.27}
\end{equation*}
$$

which are provided by an assumption that the Jacobian

$$
J=\operatorname{det}\left\|\frac{\partial\left(\xi_{1}, \xi_{2}, \xi_{3}\right)}{\partial\left(x_{1}, x_{2}, x_{3}\right)}\right\|=\left|\begin{array}{cc}
\partial \xi_{1} / \partial x_{1} & \partial \xi_{1} / \partial x_{2}  \tag{2.28}\\
\partial \xi_{1} / \partial x_{3} \\
\partial \xi_{2} / \partial x_{1} & \partial \xi_{2} / \partial x_{2} \\
\partial \xi_{2} / \partial x_{3} \\
\partial \xi_{3} / \partial x_{1} & \partial \xi_{3} / \partial x_{2} \\
\partial \xi_{3} / \partial x_{3}
\end{array}\right|
$$

exists at each point of the configuration $\mathcal{E}$ and that $J>0$ meaning that the material cannot penetrate itself, and that material element of non-zero volume cannot be compressed to a point or expanded to infinite volume during the motion. The independent variables $\mathbf{x}$ and $\boldsymbol{\xi}$ in Eq. (2.27) are referred to as Lagrange and Euler variables, respectively.

We define the strain tensor (specifying the change between the points in the deformable body) by introducing the displacement $\mathbf{u}=\boldsymbol{\xi}-\mathbf{x}, \quad u_{i}=\xi_{i}-x_{i}$ as well as the tensors of the deformation gradient $\mathbf{F}$, material displacement gradient $\mathbf{H}$, and spatial displacement gradient $(J \equiv \operatorname{det} \mathbf{F}(\mathbf{x})>0)$ :

$$
\begin{equation*}
\frac{\partial \boldsymbol{\xi}}{\partial \mathbf{x}}=\mathbf{F}(\mathbf{x}), \frac{\partial \mathbf{u}(\mathbf{x})}{\partial \mathbf{x}}=\mathbf{H}(\mathbf{x}) \equiv \mathbf{F}(\mathbf{x})-\mathbf{I}, \frac{\partial \mathbf{u}(\boldsymbol{\xi})}{\partial \boldsymbol{\xi}}=\mathbf{h}(\boldsymbol{\xi}), \mathbf{F h}=\mathbf{H} \tag{2.29}
\end{equation*}
$$

respectively. We will reproduce the correspondences between some elements of the regions $\mathcal{E}_{0}$ and $\mathcal{E}$ in the reference and current configurations, respectively. The equation $d \boldsymbol{\xi}=\mathbf{F} d \mathbf{x}$ describes how an infinitesimal line element $d \mathbf{x} \in \mathcal{E}_{0}$ transforms linearly into the element $d \boldsymbol{\xi} \in \mathcal{E}$. The areas $d s$ and $d S$ of the surface elements of $\partial \mathcal{E}$ and $\partial \mathcal{E}_{0}$ with the (positive) unit normals $\mathbf{n}$ and $\mathbf{N}$, respectively, are connected by the Nanson's formula:

$$
\begin{equation*}
\mathbf{n} d s=J \mathbf{B N} d S, \mathbf{B}=\left(\mathbf{F}^{-1}\right)^{\top} \tag{2.30}
\end{equation*}
$$

The volume $d v$ in the deformed configuration is related with the volume $d V$ in the reference configuration by the equation

$$
\begin{equation*}
d i v=J d V \tag{2.31}
\end{equation*}
$$

The ratio of current $|\boldsymbol{\xi}|$ to reference $|d \mathbf{x}|$ lengths of a line element that was in the direction $\mathbf{M}$ in the reference configuration defines the stretch:

$$
\begin{equation*}
\lambda(\mathbf{M}) \equiv \frac{|d \boldsymbol{\xi}|}{|d \mathbf{x}|}=|\mathbf{F M}|=\left[\mathbf{M} \cdot\left(\mathbf{F}^{\top} \mathbf{F M}\right)\right]^{1 / 2} \tag{2.32}
\end{equation*}
$$

in the direction $\mathbf{M}$ at $\mathbf{x}$. The quality $\lambda(\mathbf{M})-1$ is called the extension ratio in the direction $\mathbf{M}$, while $|d \boldsymbol{\xi}|-|d \mathbf{x}|$ is the extension.

The deformed states of the body in the vicinity of $P^{0}$ are defined the strain tensor specified by the displacement $\mathbf{u}$. Working with the material $\mathbf{x}$ and spatial $\boldsymbol{\xi}$ variables we can define the strain tensors in Lagrange's (Green's tensor) and Euler's (Almansi's tensor) representations:

$$
\begin{align*}
& \varepsilon=\frac{1}{2}\left(\mathbf{H}^{\top}+\mathbf{H}+\mathbf{H}^{\top} \mathbf{H}\right), \varepsilon_{i j}(\mathbf{x})=\frac{1}{2}\left(\frac{\partial u_{i}}{\partial x_{j}}+\frac{\partial u_{j}}{\partial x_{i}}+\frac{\partial u_{k}}{\partial x_{i}} \frac{\partial u_{k}}{\partial x_{j}}\right),  \tag{2.33}\\
& \tilde{\varepsilon}=\frac{1}{2}\left(\mathbf{h}^{\top}+\mathbf{h}+\mathbf{h}^{\top} \mathbf{h}\right), \tilde{\varepsilon}_{i j}(\mathbf{x})=\frac{1}{2}\left(\frac{\partial u_{i}}{\partial \xi_{j}}+\frac{\partial u_{j}}{\partial \xi_{i}}-\frac{\partial u_{k}}{\partial \xi_{i}} \frac{\partial u_{k}}{\partial \xi_{j}}\right), \tag{2.34}
\end{align*}
$$

Hereafter the values referred to the variables $\boldsymbol{\xi}$ are marked by the symbol ${ }^{\sim}$. In many practical applications, it is possible to neglect the products of derivatives in Eqs. (2.33) and (2.34). Then the coincidence of the tensors of Green and Almansi yields the well-known infinitesimal strain expression:

$$
\begin{equation*}
\varepsilon=\operatorname{Def} \mathbf{u}, \varepsilon_{i j}(\mathbf{x})=\frac{1}{2}\left(\frac{\partial u_{i}}{\partial x_{j}}+\frac{\partial u_{j}}{\partial x_{i}}\right) \tag{2.35}
\end{equation*}
$$

If a displacements $\mathbf{u}$ are given, the components of the strain tensors (2.33)-(2.35) are easily calculated by proper differentiation of these quantities. However, if six component of a strain tensor are given, they must be interrelated to serve as an integrability conditions ensuring a compatible set of the three independent displacement components. For the linear infinitesimal strain (2.35), these six interrelations, called Saint-Venan't compatibility equations, produce only three independent relations between the six components of the strain:

$$
\begin{equation*}
\text { Inc } \varepsilon=\mathbf{0}, \quad \epsilon_{i l m} \epsilon_{j p q} \frac{\partial^{2} \varepsilon_{m n}}{\partial x_{l} \partial x_{q}}=0 \tag{2.36}
\end{equation*}
$$

For an arbitrary symmetric tensor of the second order (including Green and Almansi strain tensors), the characteristic equation $\operatorname{det}\left\|\varepsilon_{i j}-\varepsilon \delta_{i j}\right\|=0$, defining a polynomial of the third degree in $\varepsilon$ :

$$
\begin{equation*}
\varepsilon^{3}-I_{\varepsilon} \varepsilon^{2}+I I_{\varepsilon} \varepsilon-I I I_{\varepsilon}=0 \tag{2.37}
\end{equation*}
$$

has three real roots $\varepsilon_{1}, \varepsilon_{2}, \varepsilon_{3}$ called principal values. The coefficients $I_{\varepsilon}, I I_{\varepsilon}, I I I_{\varepsilon}$ of Eq. (2.37) are scalars called principal basic invariants, and are given by

$$
\begin{align*}
I_{\varepsilon} & =\operatorname{tr} \varepsilon \equiv \varepsilon_{i i}=\varepsilon_{1}+\varepsilon_{2}+\varepsilon_{3}  \tag{2.38}\\
I I_{\varepsilon} & =\frac{1}{2} \epsilon_{i j k} \epsilon_{i l m} \varepsilon_{j l} \varepsilon_{k m} \equiv \frac{1}{2}\left(I_{\varepsilon}^{2}-\varepsilon: \varepsilon\right)=\varepsilon_{11} \varepsilon_{22}+\varepsilon_{22} \varepsilon_{33}+\varepsilon_{33} \varepsilon_{11} \\
& -\varepsilon_{12}^{2}-\varepsilon_{13}^{2}-\varepsilon_{23}^{3}=\varepsilon_{1} \varepsilon_{2}+\varepsilon_{2} \varepsilon_{3}+\varepsilon_{1} \varepsilon_{3}  \tag{2.39}\\
I I I_{\varepsilon} & =\operatorname{det} \varepsilon \equiv \frac{1}{6}\left(2 \varepsilon_{i j} \varepsilon_{j k} \varepsilon_{k i}-3 I_{\varepsilon} \varepsilon: \varepsilon+I_{\varepsilon}^{3}\right)=\varepsilon_{1} \varepsilon_{2} \varepsilon_{3} . \tag{2.40}
\end{align*}
$$

To each principal value $\varepsilon_{i}$, the corresponding principal direction $\mathbf{m}_{i}$ is defined by the equation ( $i$ not summed) $\boldsymbol{\varepsilon} \cdot \mathbf{m}_{i}-\varepsilon_{i} \mathbf{m}_{i}=\mathbf{0}$. The tensor $\boldsymbol{\varepsilon}$ can be expressed in diagonal form in the principal triad $\left(\mathbf{m}_{1}, \mathbf{m}_{2}, \mathbf{m}_{3}\right)$

$$
\begin{equation*}
\varepsilon=\sum_{i=1}^{3} \varepsilon_{i} \mathbf{m}_{i} \otimes \mathbf{m}_{i} \tag{2.41}
\end{equation*}
$$

which is orthogonal if $\varepsilon_{1} \neq \varepsilon_{2} \neq \varepsilon_{3}: \mathbf{m}_{i} \cdot \mathbf{m}_{j}=\delta_{i j}$. In such a case, the components of the tensor $\boldsymbol{\varepsilon}$ in the principle triad have the form (no sum on $i$ ) $\varepsilon_{i j}=\varepsilon_{i} \delta_{i j}$.

In parallel with the principle basic invariants (2.38)-(2.40) one can introduce other invariants such as, e.g., basic algebraic invariants:

$$
\begin{align*}
& A_{1}=\varepsilon_{i i}=\varepsilon_{1}+\varepsilon_{2}+\varepsilon_{3}  \tag{2.42}\\
& A_{2}=\varepsilon_{i j} \varepsilon_{j i}=\varepsilon_{1}^{2}+\varepsilon_{2}^{2}+\varepsilon_{3}^{2}  \tag{2.43}\\
& A_{3}=\varepsilon_{i j} \varepsilon_{j k} \varepsilon_{k i}=\varepsilon_{1}^{3}+\varepsilon_{2}^{3}+\varepsilon_{3}^{3} \tag{2.44}
\end{align*}
$$

as well as the invariant system specified by the intensity $e_{i}$ of the tensor deviator $\varepsilon^{\prime}$ and its phase $\psi$

$$
\begin{align*}
e & =\left(\varepsilon_{1}+\varepsilon_{2}+\varepsilon_{3}\right) / 3  \tag{2.45}\\
e_{i} & =\frac{1}{3} \sqrt{\left(\varepsilon_{1}-\varepsilon_{2}\right)^{2}+\left(\varepsilon_{2}-\varepsilon_{3}\right)^{2}+\left(\varepsilon_{1}-\varepsilon_{3}\right)^{2}},  \tag{2.46}\\
\frac{1}{\sqrt{2}} \cos 3 \psi & =e_{i}^{-3}\left(\varepsilon_{1}-e\right)\left(\varepsilon_{2}-e\right)\left(\varepsilon_{3}-e\right) . \tag{2.47}
\end{align*}
$$

The following relationships between the invariants hold true:

$$
\begin{align*}
I_{\varepsilon} & =A_{1}, 2 I I_{\varepsilon}=a_{1}^{2}-A_{2}, I I I_{\varepsilon}=\left(2 A_{3}-3 A_{2} A_{1}+A_{1}^{3}\right) / 6,  \tag{2.48}\\
e & =\frac{1}{3} A_{1}, e_{i}=\frac{1}{3} \sqrt{3 A_{2}-A_{1}^{2}}, \\
\cos 3 \psi & =\sqrt{2}\left(9 A_{3}-9 A_{1} A_{2}+2 A_{1}^{3}\right)\left(3 A_{2}-A_{1}^{2}\right)^{3 / 2} \tag{2.49}
\end{align*}
$$

Now we will consider some definitions and notations of stress analysis. Let an element of the internal surface with an area $d s$ have a unit normal vector $\mathbf{n}$. The total force $d \mathbf{f}$ acting on this element of surface defines the surface traction $\mathbf{t}^{(n)}=$ $d \mathbf{f} / d s$, where the superscript $(n)$ designates the normal to the surface element. The second-order Cauchy (or Eulerian) stress tensor $\mathbf{T}$ is formally related to the stress traction by the symmetric tensor equation:

$$
\begin{equation*}
\mathbf{t}^{(n)}=\mathbf{n} \cdot \mathbf{T}, \quad t_{j}^{(n)}=n_{i} T_{i j} . \tag{2.50}
\end{equation*}
$$

The tensor $\mathbf{T}$ is symmetric if and only if balance of moment of momentum holds. Then the stress state in the point $\boldsymbol{\xi}$ can be described either by six components $T_{i j}$ of the tensor $\mathbf{T}$ or by three invariants $I_{T}, I I_{T}, I I I_{T}$ called the principal stresses [introduced analogously to Eqs. (2.38)-(2.40), $\mathbf{T}=\boldsymbol{\sigma}$ ]:

$$
\begin{align*}
I_{\sigma} & =\operatorname{tr} \boldsymbol{\sigma} \equiv \sigma_{i i}=\sigma_{1}+\sigma_{2}+\sigma_{3}  \tag{2.51}\\
I I_{\sigma} & =\frac{1}{2} \epsilon_{i j k} \epsilon_{i l m} \sigma_{j l} \sigma_{k m} \equiv \frac{1}{2}\left(I_{\sigma}^{2}-\boldsymbol{\sigma}: \boldsymbol{\sigma}\right)=\sigma_{11} \sigma_{22}+\sigma_{22} \sigma_{33}+\sigma_{33} \sigma_{11} \\
& -\sigma_{12}^{2}-\sigma_{13}^{2}-\sigma_{23}^{3}=\sigma_{1} \sigma_{2}+\sigma_{2} \sigma_{3}+\sigma_{1} \sigma_{3}  \tag{2.52}\\
I I I_{\sigma} & =\operatorname{det} \boldsymbol{\sigma} \equiv \frac{1}{6}\left(2 \sigma_{i j} \sigma_{j k} \sigma_{k i}-3 I_{\sigma} \boldsymbol{\sigma}: \boldsymbol{\sigma}+I_{\sigma}^{3}\right)=\sigma_{1} \sigma_{2} \sigma_{3} \tag{2.53}
\end{align*}
$$

and by the Eulerian angles specifying the principle triad.
The Cauchy stress tensor defined by Eq. (2.50) refers to the deformed body in the spatial coordinate system. In a similar manner, we can introduce a nominal stress tensor (that is transposed of the first Piola-Kirchhoff stress tensor) associated with an original element of undeformed area:

$$
\begin{equation*}
\mathbf{T}^{0}=J \mathbf{F}^{-1} \mathbf{T}, \quad T_{i j}^{0}=J \frac{\partial x_{i}}{\partial \xi_{k}} T_{k j}, \tag{2.54}
\end{equation*}
$$

where the Jacobian $J$ is written as a determinant (2.28). The first Piola-Kirchhoff stress tensor is a two-point and, in general, nonsymmetric tensor. For elimination of these two disadvantages, one introduces the symmetric second Piola-Kirchhoff stress tensor

$$
\begin{equation*}
\boldsymbol{\sigma}=\mathbf{T}^{0} \mathbf{B}=J \mathbf{B}^{\top} \mathbf{T B}, \quad \sigma_{i j}=T_{i m}^{0} \frac{\partial x_{j}}{\partial \xi_{m}}=J \frac{\partial x_{i}}{\partial \xi_{m}} T_{m n} \frac{\partial x_{j}}{\partial \xi_{n}}, \tag{2.55}
\end{equation*}
$$

which may be inverted

$$
\begin{equation*}
T_{i j}=J^{-1} \frac{\partial \xi_{i}}{\partial x_{m}} \sigma_{m n} \frac{\partial \xi_{i}}{\partial x_{n}}, T_{i m}^{0}=\sigma_{i j} \frac{\partial \xi_{m}}{\partial x_{j}} . \tag{2.56}
\end{equation*}
$$

### 2.3 Basic Equations of Solid Mechanics

### 2.3.1 Conservation Laws, Boundary Conditions, and Constitutive <br> \section*{Equation}

There are four conservation laws in continuum mechanics: (1) mass conservation, (2) conservation of linear momentum, (3) conservation of angular momentum, and (4) conservation of energy. We will reproduce the first, second, and fourth conservation laws.

The balance equation of linear momentum in spatial coordinates in terms of the Cauchy stress tensor $\mathbf{T}$ is

$$
\begin{equation*}
\frac{\partial T_{i j}}{\partial \xi_{i}}+\rho b_{i}=\rho\left(\frac{\partial v_{i}}{\partial t}+v_{i} \frac{\partial v_{j}}{\partial \xi_{i}}\right), \tag{2.57}
\end{equation*}
$$

where $\mathbf{b}$ is the body force per unit of mass acting upon the volume element; $\rho$ is the mass density at time $t$ related to the initial mass density $\rho_{0}$ by the equation of continuity (the first conservation law):

$$
\begin{equation*}
\rho=J^{-1} \rho_{0} \tag{2.58}
\end{equation*}
$$

according to Eq. (2.31). $v_{i}$ is a velocity of a particle occupying the coordinate $\xi_{i}$ at some time $t$ found by taking the time derivative and holding the material coordinates constant: $v_{i}=\partial \xi_{i} / \partial t$.

Since the boundary conditions in elastic problems are most easily expressed in material coordinates, we shall reproduce an equivalent equation of motion in

Lagrangian coordinates in terms of the first and second Piola-Kirchhiff stress tensors:

$$
\begin{align*}
\frac{\partial T_{i m}^{0}}{\partial x_{i}}+\rho_{0} b_{0 m} & =\rho_{0} \frac{d^{2} u_{m}(\mathbf{x})}{d t^{2}}  \tag{2.59}\\
\frac{\partial}{\partial x_{i}}\left[\sigma_{i j}\left(\delta_{j m}+\frac{\partial u_{m}(\mathbf{x})}{\partial x_{j}}\right)\right]+\rho_{0} b_{0 m} & =\rho_{0} \frac{d^{2} u_{m}(\mathbf{x})}{d t^{2}} \tag{2.60}
\end{align*}
$$

respectively, where the time derivative is actually a simple time derivative.
The basic energy-conservation equation (or first principle of thermodynamic) equates the rate of change of kinetic $K$ and internal energy $E$ to the rate at which surface and body forces do mechanical work $P$ and the rate at which nonmechanical energy $Q$ is transferred (per unit volume):

$$
\begin{equation*}
\dot{K}+\dot{E}=\dot{Q}+P \tag{2.61}
\end{equation*}
$$

where

$$
\begin{align*}
K & =\frac{1}{2 \overline{\mathcal{E}}} \int_{\mathcal{E}} \rho v_{i} v_{i} d v, \quad E=\frac{1}{\overline{\mathcal{E}}} \int_{\mathcal{E}} \rho e d v  \tag{2.62}\\
Q & =-\frac{1}{\overline{\mathcal{E}}} \int_{\partial \mathcal{E}} q_{k} n_{k} d s+\frac{1}{\overline{\mathcal{E}}} \int_{\mathcal{E}} h d v=\frac{1}{\overline{\mathcal{E}}} \int_{\mathcal{E}}(-\nabla \cdot \mathbf{q}+h) d v  \tag{2.63}\\
P & =\frac{1}{\overline{\mathcal{E}}} \int_{\mathcal{E}} b_{i} v_{i} d v+\frac{1}{\overline{\mathcal{E}}} \int_{\partial \mathcal{E}} T_{i j} n_{j} v_{i} d s \tag{2.64}
\end{align*}
$$

where $e$ is a specific internal energy per unit mass, $\mathbf{q}$ is a nonmechanical energy flux tensor defining the rate $q_{k} n_{k} d s$ of any nonmechanical energy transmitted outward through the surface element $d s$ with outward directed normal $\mathbf{n}$, and $h$ is heat created per unit volume in the body. In Eq. (2.61) $K, E, Q$, and $P$ are global quantities represented as integrals over the volume and its surface (2.62)-(2.64), but they also have local analogies (for which we will use the same notations) defined pointwise within the body. Equation (2.61) can be transformed into the differential form by the use of the equation of continuity (2.58) and the equations of motion:

$$
\begin{equation*}
\rho \frac{d e}{d t}=T_{i j} d_{i j}-\frac{\partial q_{j}}{\partial \xi_{j}}+\rho h, \quad d_{i j}=\frac{1}{2}\left(\frac{\partial v_{j}}{\partial \xi_{i}}+\frac{\partial v_{i}}{\partial \xi_{j}}\right) \tag{2.65}
\end{equation*}
$$

where, $\mathbf{d}$ called the rate strain tensor, is the symmetric part of the tensor $v_{i, j}$, and $d / d t=\partial / \partial t+v_{i} \partial / \partial \xi_{i}$ is a material derivative.

The second law of thermodynamics states the existence of the total differential for reversible processes:

$$
\begin{equation*}
d S=\frac{d Q}{T} \tag{2.66}
\end{equation*}
$$

where $S$ is the entropy regarded as a measure of energy dissipation with respect to the absolute temperature $T$, and

$$
\begin{equation*}
\dot{S}+\int_{\partial \mathcal{E}}\left(\frac{\mathbf{q}}{T}\right) \cdot \mathbf{n} d s-\int_{\mathcal{E}} \frac{h}{T} d v \geq 0 \tag{2.67}
\end{equation*}
$$

The last expression is referred to as the second law of thermodynamics, known as the Clausius-Duhem inequality, which can be rewritten as

$$
\begin{equation*}
\int_{\mathcal{E}}\left[\dot{s}+\nabla \cdot\left(\frac{\mathbf{q}}{T}\right)-\frac{h}{T}\right] d i v \geq 0 \tag{2.68}
\end{equation*}
$$

or

$$
\begin{equation*}
T \dot{s}+\nabla \cdot \mathbf{q}-\frac{1}{T} \mathbf{q} \cdot \nabla T-h \geq 0 \tag{2.69}
\end{equation*}
$$

which is the local form of the Clausius-Duhem inequality. Here and henceforth $s$ is the specific entropy density, and the term "specific" will mean "per unit volume". Without loss of generality, it is assumed mes $\mathcal{E}=1$.

If the right-hand side of motion equations (2.57), (2.59), (2.60) are zero, the resulting equations are called the equilibrium equation. One way to maintain a body in equilibrium is to apply suitable boundary conditions. The typical boundary conditions involves the prescribed displacements $\mathbf{u}$ specifying on part of the boundary $\partial \mathcal{E}_{\mathbf{u}}^{0} \subset \partial \mathcal{E}^{0}$, and the stress vector on the remained, $\partial \mathcal{E}_{\sigma}^{0}$, so that

$$
\begin{align*}
\mathbf{u}(\mathbf{x}) & =\mathbf{u}^{\partial}(\mathbf{x}),  \tag{2.70}\\
\text { on } & \partial \mathcal{E}_{\mathbf{u}}^{0}  \tag{2.71}\\
\mathbf{T}^{0 \top} \cdot \mathbf{N} & =\mathbf{t}^{0(\mathbf{N})}(\mathbf{F}, \mathbf{x})
\end{aligned} \begin{aligned}
& \text { on } \partial \mathcal{E}_{\sigma}^{0},
\end{align*}
$$

where $\partial \mathcal{E}_{\mathbf{u}}^{0} \cap \partial \mathcal{E}_{\sigma}^{0}=\partial \mathcal{E}^{0}, \partial \mathcal{E}_{\mathbf{u}}^{0} \cup \partial \mathcal{E}_{\sigma}^{0}=0$. A general configuration dependent loading (2.70) if reduced to a dead-load traction in the surface traction introduced by (2.70) is independent of $\mathbf{F}$.

The aforementioned equations of conservation laws are in general insufficient to determine the body motion produced by given boundary conditions and body forces. They need to be accompanied by a constitutive equation characterizing the constitution of the body. It is assumed that there are no stresses and no strains in the initial (virgin) strain of the body. For Cauchy elastic materials, it is described by a symmetric single-valued response function $\mathbf{G}$ : $\mathbf{T}=\mathbf{G}(\mathbf{F})$, which does not depend on the path of deformation. In a special case of Cauchy elasticity called hyperelasticity, there exists a specific strain-energy function $w=w(\mathbf{F})$ defined on the space of deformation gradients:

$$
\begin{equation*}
\mathbf{T}^{0}=\frac{\partial w}{\partial \mathbf{F}}, \quad \mathbf{T}=J^{-1} \mathbf{F} \frac{\partial w}{\partial \mathbf{F}} \tag{2.72}
\end{equation*}
$$

Equations (2.72) are the constitutive equations for finite-deformation elasticity. The function $w$ defined per unit volume in $\mathcal{E}^{0}$ represents the work done per unit volume at $\mathbf{x} \in \mathcal{E}^{0}$ in changing the deformation gradient from $\mathbf{I}$ to $\mathbf{F}$. The function $w$ can be considered as depending on either $\boldsymbol{\varepsilon}$ (2.33) or $H_{i j}(2.29)$. Then we get the representations

$$
\begin{equation*}
\sigma_{i j}=\frac{\partial w(\boldsymbol{\varepsilon})}{\varepsilon_{i j}}, \quad T_{i j}^{0}=\frac{\partial w(\mathbf{H})}{H_{i j}} \tag{2.73}
\end{equation*}
$$

relating the second Piola-Kirchhoff stress tensor $\boldsymbol{\sigma}$ to the Green deformation tensor $\varepsilon$, and the nominal stress tensor $\mathbf{T}^{0}$ to the material displacement gradient $\mathbf{H}$, respectively.

A material is said to be isotropic if there is no orientation effect in the material. In such a case, the strain energy can be presented either as a symmetric function of the principal stretches or as a function of three independent invariants, such as $(2.38)-(2.40)$ or (2.42)-(2.44). For an incompressible material
$J=I I I_{\varepsilon} \equiv 1$ holds, and the function $w$ depends on only two independent invariants. In particular, the Mooney-Rivlind form of strain energy

$$
\begin{equation*}
w=C_{1}\left(I_{\varepsilon}-3\right)+C_{2}\left(I I_{\varepsilon}-3\right),\left(C_{1}, C_{2}=\text { const. }\right) \tag{2.74}
\end{equation*}
$$

is reduced to the neo-Hookean form

$$
\begin{equation*}
w=C_{1}\left(I_{\varepsilon}-3\right) \tag{2.75}
\end{equation*}
$$

Using Eq. $\left(2.73_{1}\right)$ and the forms of the strain-energy functions (2.74) and (2.75), the stress tensor take the forms $\boldsymbol{\sigma}=-p \mathbf{I}+2 C_{1} \mathbf{F} \mathbf{F}^{\top}$ and $\boldsymbol{\sigma}=-p \mathbf{I}+2 C_{1} \mathbf{F F}^{\top}$ $2 C_{2}\left(\mathbf{F F}^{\top}\right)^{-1}$ for Mooney-Rivlin and neo-Hookean materials, respectively; here $p$ is an arbitrary scalar. Many other forms of the strain energy have been analyzed for both incompressible and compressible nonlinear rubberlike solids in [26], [830].

Now we will consider the strain-energy functions of physically nonlinear elasticity defined on the space of infinitesimal strain tensors (2.35). A general quadratic form of the function $w$ composed by the algebraic invariants (2.42) and (2.43) is

$$
\begin{equation*}
w=a_{1} A_{1}^{2}+a_{2} A_{2} \tag{2.76}
\end{equation*}
$$

leading to the known constitutive equation $\left(2.73_{1}\right)$ for the isotropic medium:

$$
\begin{equation*}
\boldsymbol{\sigma}=\mathbf{L} \boldsymbol{\varepsilon}, L=3 \lambda \mathbf{N}_{1}+2 \mu \mathbf{I} \tag{2.77}
\end{equation*}
$$

by the use of two, $\lambda$ and $\mu$, Lame's elastic constants. A more general special form of specific strain energy depending on three basic algebraic invariants (2.42)(2.44) is the Murnaghan form, defined by

$$
\begin{equation*}
w=\frac{1}{2} \lambda A_{1}^{2}+\mu A_{2}+\frac{a}{3} A_{1}^{3}+b A_{1} A_{2}+\frac{c}{3} A_{3}, \tag{2.78}
\end{equation*}
$$

where $\lambda$ and $\mu$ are the Lame' elastic constants of the second order, and $a, b, c$ are the elastic constants of the third order. Equations (2.731) and (2.78) yield the following tensor constitutive equations:

$$
\begin{align*}
\boldsymbol{\sigma} & =\mathbf{L} \varepsilon+\mathbf{L}^{(3)} \varepsilon \otimes \varepsilon  \tag{2.79}\\
L_{i j k l} & =\lambda \delta_{i j} \delta_{k l}+\mu\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right)  \tag{2.80}\\
L_{i j k l m n} & =a \delta_{i j} \delta_{m n} \delta_{k l}+b\left(\delta_{i j} I_{m n k l}+\delta_{m n} I_{i j k l}+\delta_{k l} I_{m n i j}\right)+c J_{i j m n k l},  \tag{2.81}\\
J_{i j m n k l} & =\frac{1}{2}\left(I_{i p k l} I_{p q m n}+I_{i p m n} I_{p j k l}\right) . \tag{2.82}
\end{align*}
$$

The Kauderer potential depending on two basic algebraic invariants (2.42)-(2.43) and three constant $(\lambda, \mu, \gamma)$ is known:

$$
\begin{equation*}
w=\frac{1}{2} \lambda A_{1}^{2}+\mu A_{2}+\frac{1}{3} \gamma\left(A_{2}-\frac{1}{3} A_{1}\right)^{2} . \tag{2.83}
\end{equation*}
$$

The strain energy (2.83) is associated with constitutive equations

$$
\begin{align*}
\sigma_{i j} & =\lambda \varepsilon_{k k} \delta_{i j}+2 \mu \varepsilon_{i j}+2 \gamma \varepsilon_{\mathrm{eq}}\left(\varepsilon_{i j}-\frac{1}{3} \delta_{i j} \varepsilon_{k k}\right),  \tag{2.84}\\
\varepsilon_{i j} & =\frac{1}{9 k} \sigma_{k k} \delta_{i j}+\frac{1}{2 \mu}\left(1-\frac{\gamma}{9 \mu^{3}} \tau^{2}\right) s_{i j}, \tag{2.85}
\end{align*}
$$

where $k \equiv \lambda+2 \mu / 3$ is the bulk modulus, and $\tau$ and $\varepsilon_{\text {eq }}$ are the stress and strain intensities

$$
\begin{equation*}
\tau=\left(\frac{3}{2} \mathbf{s}: \mathbf{s}\right)^{1 / 2}, \quad \varepsilon_{\mathrm{eq}}=\left(\frac{2}{3} \mathbf{e}: \mathbf{e}\right)^{1 / 2} \tag{2.86}
\end{equation*}
$$

respectively, where $\mathbf{s}=\mathbf{N}_{2} \boldsymbol{\sigma}$ and $\mathbf{e}=\mathbf{N}_{2} \boldsymbol{\varepsilon}$. The constitutive equation (2.84) can be presented in more compact form:

$$
\begin{equation*}
\boldsymbol{\sigma}=\mathbf{L} \varepsilon+\boldsymbol{\Psi}(\varepsilon), \mathbf{L}=3 k \mathbf{N}_{1}+2 \mu \mathbf{N}_{2}, \quad \Psi=2 \gamma \varepsilon_{\mathrm{eq}}^{2} \mathbf{N}_{2} \tag{2.87}
\end{equation*}
$$

where the tensor $\boldsymbol{\Psi}$ depending on the effective strains $\varepsilon_{\text {eq }}$ describes the nonlinear material properties.

Also used is an asymptotic expansion of the specific strain energy in a Taylor series about the state of zero strain and stress as

$$
\begin{equation*}
w=\frac{1}{2} L_{i j k l}^{(2)} \varepsilon_{i j} \varepsilon_{k l}+\frac{1}{6} L_{i j k l m n}^{(3)} \varepsilon_{i j} \varepsilon_{k l} \varepsilon_{m n}+\ldots \tag{2.88}
\end{equation*}
$$

assuming $w(\mathbf{0})=0$. One can get from Eq. (2.88) that

$$
\begin{equation*}
\mathbf{L}^{(n)}=\frac{\partial^{(n)} w}{\partial \boldsymbol{\varepsilon}^{(n)}} \tag{2.89}
\end{equation*}
$$

where $n=2,3, \ldots$ and $\varepsilon^{(n)}=\varepsilon \otimes \ldots \otimes \varepsilon$ is $n$-multiple tensor production of the tensor $\boldsymbol{\varepsilon}$. The equality (2.89) yields the following symmetry properties of the elasticity tensors $\mathbf{L}^{(n)}$ of the $2 n$ order with respect to the index pairs

$$
\begin{align*}
L_{i j k l}^{(2)} & =L_{j i k l}^{(2)}=L_{i j l k}^{(2)}=L_{l k i j}^{(2)}  \tag{2.90}\\
L_{i j k l p q}^{(3)} & =L_{j i k l p q}^{(3)}=L_{i j l k p q}^{(3)}=L_{i j k l q p}^{(3)}=L_{k l i j p q}^{(3)}=L_{p q k l i j}^{(3)}=L_{i j p q k l}^{(3)} \tag{2.91}
\end{align*}
$$

and so forth. In so doing, the "pair symmetry" described by two first equalities of Eq. (2.90) and corresponding to an interchange of the indices of the first pair and interchange of the indices of the last pair are followed from the symmetry of both the $\boldsymbol{\sigma}$ and $\boldsymbol{\varepsilon}$ tensors. The "diagonal symmetry" corresponding to an interchange of the first pair of indices with the second one is described by the third equality of Eq. (2.90) and defined by Eq. (2.88). Fourth-order tensors with pair and diagonal-symmetry are referred as "full-symmetric" tensors.

### 2.3.2 The Equations of Linear Elasticity

We will reproduce the reduction of the elasticity theory of finite deformation to a linear theory, which is a special case of small deformations superposed on a finite deformations with the special values $\mathbf{b}_{0}=\mathbf{0}, \mathbf{F}_{0}=\mathbf{I}, \mathbf{T}_{0}^{0}=\mathbf{0}$. Then all stress tensors coincide and can be recognized as a Cauchy stress. All strain tensors likewise reduce to the infinitesimal strain tensor (2.35). Then the equalities

$$
\begin{equation*}
\boldsymbol{\sigma}=\mathbf{T}=\mathbf{T}^{0}, \quad \varepsilon=\tilde{\varepsilon}, \quad J=1+\varepsilon_{i i} \tag{2.92}
\end{equation*}
$$

simplify the basic equations of elasticity theory

$$
\begin{align*}
\nabla \boldsymbol{\sigma}+\rho \mathbf{b} & =\rho \ddot{\mathbf{u}}, \quad \frac{\partial \sigma_{i j}}{\partial x_{j}}+\rho b_{i}=\rho \frac{\partial^{2} u_{i}}{\partial t^{2}}  \tag{2.93}\\
\boldsymbol{\sigma} & =\mathbf{L} \boldsymbol{\varepsilon}, \quad \sigma_{i j}=L_{i j k l} \varepsilon_{k l}  \tag{2.94}\\
\boldsymbol{\varepsilon} & =\text { Defu, } \quad \varepsilon_{i j}=\frac{1}{2}\left(\frac{\partial u_{i}}{\partial x_{j}}+\frac{\partial u_{j}}{\partial x_{i}}\right) . \tag{2.95}
\end{align*}
$$

The mixed boundary conditions on $\partial \mathcal{E}$ with the unit outward normal $\mathbf{n}^{\partial \mathcal{E}}$ will be considered

$$
\begin{align*}
\mathbf{u}(\mathbf{x}) & =\mathbf{u}^{\partial \mathcal{E}}(\mathbf{x}), & \mathbf{x} \in \partial \mathcal{E}_{\mathbf{u}}  \tag{2.96}\\
\boldsymbol{\sigma}(\mathbf{x}) \cdot \mathbf{n}^{\partial \mathcal{E}}(\mathbf{x}) & =\mathbf{t}^{\partial \mathcal{E}}(\mathbf{x}), & \mathbf{x} \in \partial \mathcal{E}_{\sigma} \tag{2.97}
\end{align*}
$$

where $\partial \mathcal{E}_{\mathbf{u}}$ and $\partial \mathcal{E}_{\sigma}$ are prescribed displacement and traction non-intersected boundary conditions such that $\partial \mathcal{E}_{\mathbf{u}} \cup \partial \mathcal{E}_{\sigma}=\partial \mathcal{E} ; \mathbf{u}^{\partial \mathcal{E}}(\mathbf{x})$ and $\mathbf{t}^{\partial \mathcal{E}}(\mathbf{x})$ are prescribed the displacement on $\partial \mathcal{E}_{\mathbf{u}}$ and traction on $\partial \mathcal{E}_{\sigma}$, respectively; mixed boundary conditions, such as in the case of elastic supports are also possible. When $\partial \mathcal{E}_{\sigma}$ is empty, the mixed boundary conditions (2.96) and (2.97) reduce to the displacement problem or the first boundary value problem. If $\partial \mathcal{E}_{\mathbf{u}}$ is empty, the boundary conditions (2.96), (2.97) becomes the traction problem or the second boundary value problem. As usual we shall distinguish the interior from the exterior problem according to whether the body occupies the interior or the exterior domain with respect to $\mathcal{E}$.

Of special practical interest are the homogeneous boundary conditions:

$$
\begin{align*}
\mathbf{u}^{\partial \mathcal{E}}(\mathbf{x}) & =\varepsilon^{\partial \mathcal{E}} \mathbf{x}, \boldsymbol{\varepsilon}^{\partial \mathcal{E}}(\mathbf{x}) \equiv \text { const., } \mathbf{x} \in \partial \mathcal{E}  \tag{2.98}\\
\boldsymbol{\sigma}(\mathbf{x}) \cdot \mathbf{n}^{\partial \mathcal{E}}(\mathbf{x}) & =\boldsymbol{\sigma}^{\partial \mathcal{E}}(\mathbf{x}) \cdot \mathbf{n}^{\partial \mathcal{E}}(\mathbf{x}), \quad \boldsymbol{\sigma}^{\partial \mathcal{E}}(\mathbf{x})=\text { const. }, \mathbf{x} \in \partial \mathcal{E} \tag{2.99}
\end{align*}
$$

where $\varepsilon^{\partial \mathcal{E}}(\mathbf{x})=\frac{1}{2}\left[\nabla \otimes \mathbf{u}^{\partial \mathcal{E}}(\mathbf{x})+\left(\nabla \otimes \mathbf{u}^{\partial \mathcal{E}}(\mathbf{x})\right)^{\top}\right], \mathbf{x} \in \partial \mathcal{E}_{\mathbf{u}}$.

### 2.3.3 Extremum Principles of Elastostatic

The principle of minimum potential energy and the principle of minimum complementary energy completely characterize the solution of the mixed boundary problem (2.93)-(2.97) of elastostatic ( $\ddot{\mathbf{u}} \equiv \mathbf{0}$ in Eq. (2.93)). For definition of these energy functions we introduce the notion of admissible fields. A sufficiently smooth (of class $C^{1}$ ) displacement field $\mathbf{u}$ with the displacement boundary conditions:

$$
\begin{equation*}
\mathcal{A}^{k}(\mathbf{u})=\left\{\mathbf{u} \mid \mathbf{u}(\mathbf{x}) \in C^{1}(\mathcal{E}), \mathbf{u}(\mathbf{y})=\mathbf{u}^{\partial \mathcal{E}}(\mathbf{y}), \mathbf{y} \in \partial \mathcal{E}_{\mathbf{u}}\right\} \tag{2.100}
\end{equation*}
$$

is called a kinematically admissible displacement field. A kinematically admissible state is an ordered array $s=\{\mathbf{u}, \boldsymbol{\varepsilon}, \boldsymbol{\sigma}\}$ (forming the set $\mathcal{A}^{k}(s)$ ) of kinematically admissible displacement field $\mathbf{u} \in \mathcal{A}^{k}(\mathbf{u})(2.100)$ generating the symmetric fields
$\boldsymbol{\varepsilon}$ and $\boldsymbol{\sigma}$ according to Eqs. (2.95) and (2.94), respectively. While $\boldsymbol{\varepsilon}(\mathbf{u})$ is compatible, $\boldsymbol{\sigma}(\mathbf{u})$ may not necessarily satisfy the equilibrium equations (2.93).

A sufficiently smooth (of class $C^{1}$ ) stress field satisfying the equilibrium equation (2.93) and the traction boundary condition (2.97):

$$
\begin{equation*}
\mathcal{A}^{s}(\boldsymbol{\sigma})=\left\{\boldsymbol{\sigma} \mid \boldsymbol{\sigma}(\mathbf{x}) \in C^{1}(\mathcal{E}), \nabla \cdot \boldsymbol{\sigma}+\rho \mathbf{b}=\mathbf{0}, \boldsymbol{\sigma}(\mathbf{y}) \cdot \mathbf{n}^{\partial \mathcal{E}}(\mathbf{y})=\mathbf{t}^{\partial \mathcal{E}}(\mathbf{y}), \mathbf{y} \in \partial \mathcal{E}_{\sigma}\right\} \tag{2.101}
\end{equation*}
$$

is referred to as a statically admissible stress field. The ordered array $s^{s}=$ $\{\mathbf{u}, \boldsymbol{\varepsilon}, \boldsymbol{\sigma}\}$ (forming the set $\mathcal{A}^{s}(s)$ ) is called a statically admissible state if $\boldsymbol{\sigma} \in$ $\mathcal{A}^{s}(\boldsymbol{\sigma})(2.101)$ generates the symmetric tensor $\boldsymbol{\varepsilon}$ according to the constitutive equation (2.94) and the displacement field $\mathbf{u}$ is related to $\varepsilon$ by the use of Eq. (2.95). In so doing, $\boldsymbol{\varepsilon}(\boldsymbol{\sigma})$ is not necessarily compatible.

We also define the total strain energy $W(\boldsymbol{\varepsilon})$ and the total stress energy $W^{c}(\boldsymbol{\sigma})$ by

$$
\begin{equation*}
W(\varepsilon)=\frac{1}{2} \int_{\mathcal{E}} \varepsilon: \mathbf{L}: \boldsymbol{\varepsilon} d \mathbf{x}, \quad W^{c}(\boldsymbol{\sigma})=\frac{1}{2} \int_{\mathcal{E}} \boldsymbol{\sigma}: \mathbf{M}: \boldsymbol{\sigma} d \mathbf{x} \tag{2.102}
\end{equation*}
$$

respectively. Let $\delta \mathbf{u}=\delta \mathbf{u}(\mathbf{x})$ be a virtual, or imaginative, infinitesimal displacement field associated with a virtual strain field from the current state $\delta \varepsilon=\operatorname{Def}(\delta \mathbf{u})$. The components of the virtual displacement vector $\delta \mathbf{u}$ vanish at $\partial \mathcal{E}_{\mathbf{u}}: \delta \mathbf{u}(\mathbf{y}) \equiv \mathbf{0}$ at $\mathbf{y} \in \partial \mathcal{E}_{\mathbf{u}}$. It can be shown from the equations of linear and angular momentum, that it holds the principle of virtual work (PVW):

$$
\begin{equation*}
\int_{\mathcal{E}} \boldsymbol{\sigma}: \delta \varepsilon d \mathbf{x}=\int_{\partial \mathcal{E}} \mathbf{t}^{\partial \mathcal{E}_{\sigma}} \cdot \delta \mathbf{u} d \mathbf{s}+\int_{\mathcal{E}} \rho \mathbf{b} \cdot \delta \mathbf{u} d \mathbf{x} \tag{2.103}
\end{equation*}
$$

which is a starting point in developing minimum principles.
As a consequence, one can obtain a work-energy relation (theorem of work expended) involving the admissible stress field $\boldsymbol{\sigma}$ and the admissible displacement field $\mathbf{u}[\varepsilon(\mathbf{u})$ is the related strain field] corresponding to the given external forces $\rho \mathbf{b}(\mathbf{x})$ and $\mathbf{t}^{\partial \mathcal{E}_{\sigma}}(\mathbf{y})\left(\mathbf{x} \in \mathcal{E}, \mathbf{y} \in \partial \mathcal{E}_{\sigma}\right):$

$$
\begin{equation*}
\int_{\mathcal{E}} \boldsymbol{\sigma}: \varepsilon d \mathbf{x}=\int_{\partial \mathcal{E}} \mathbf{t}^{\partial \mathcal{E}_{\sigma}} \cdot \mathbf{u} d \mathbf{s}+\int_{\mathcal{E}} \rho \mathbf{b} \cdot \mathbf{u} d \mathbf{x} \tag{2.104}
\end{equation*}
$$

which looks similar to the first law of thermodynamics (2.61), although there is no thermodynamic expression in (2.104). We note that $\boldsymbol{\sigma}$ and $\boldsymbol{\varepsilon}$ need not be connected by a specific stress-strain relation.

We will present the classical extremum principles.
Principle of minimum of potential energy. Let $\Pi$, called the potential energy (corresponding to the given external forces $\rho \mathbf{b}(\mathbf{x})$ and $\mathbf{t}^{\partial \mathcal{E}_{\sigma}}(\mathbf{y}), \mathbf{x} \in \mathcal{E}, \mathbf{y} \in$ $\partial \mathcal{E}_{\sigma}$ ), be the function defined on the set of kinematically admissible states $\tilde{s}^{k}=$ $\{\tilde{\mathbf{u}}, \tilde{\varepsilon}, \tilde{\boldsymbol{\sigma}}\} \in \mathcal{A}^{k}(\tilde{s})$ by the equation

$$
\begin{equation*}
\Pi\left(\tilde{s}^{k}\right)=W(\tilde{\varepsilon})-\int_{\mathcal{E}} \rho \mathbf{b} \cdot \tilde{\mathbf{u}} d \mathbf{x}-\int_{\partial \mathcal{E}_{\sigma}} \mathbf{t}^{\partial \mathcal{E}_{\sigma}} \cdot \tilde{\mathbf{u}} d \mathbf{s} \tag{2.105}
\end{equation*}
$$

Then the actual displacement field $\mathbf{u}$ with corresponding admissible state $s^{k}$ renders the potential energy $\Pi$ an absolute minimum

$$
\begin{equation*}
\Pi\left(s^{k}\right) \leq \Pi\left(\tilde{s}^{k}\right) \tag{2.106}
\end{equation*}
$$

Principle of minimum of complementary energy. Let $\Pi^{c}$ called the complementary energy (corresponding to the given boundary displacement $\mathbf{u}^{\partial \mathcal{E}_{\mathbf{u}}}(\mathbf{y}) \in \partial \mathcal{E}_{\mathbf{u}}$ ) be the function defined on the set of statically admissible states $\tilde{s}^{s}=\{\tilde{\mathbf{u}}, \tilde{\varepsilon}, \tilde{\boldsymbol{\sigma}}\} \in$ $\mathcal{A}^{s}(\tilde{s})$ by the equation

$$
\begin{equation*}
\Pi^{c}\left(\tilde{s}^{k}\right)=W^{c}(\tilde{\boldsymbol{\sigma}})-\int_{\partial \mathcal{E}_{\mathbf{u}}} \tilde{\mathbf{t}}_{\mathbf{n}} \cdot \mathbf{u}^{\partial \mathcal{E}_{\mathbf{u}}} d \mathbf{s}, \tilde{\mathbf{t}}_{\mathbf{n}} \equiv \tilde{\boldsymbol{\sigma}} \cdot \mathbf{n}^{\partial \mathcal{E}} \tag{2.107}
\end{equation*}
$$

Then the actual stress field $\boldsymbol{\sigma}$ with corresponding admissible state $s^{s}$ renders the complementary energy $\Pi^{c}$ an absolute minimum

$$
\begin{equation*}
\Pi^{c}\left(s^{s}\right) \leq \Pi^{c}\left(\tilde{s}^{s}\right) \tag{2.108}
\end{equation*}
$$

### 2.4 Basic Equations of Thermoelasticity and Electroelasticity

### 2.4.1 Thermoelasticity Equations

In classical thermodynamics we are concerned with the small neighborhood of thermodynamic equilibrium. We also consider the infinitesimal deformations (2.92). In such a case the first law of thermodynamics (2.65) can be recast in the form (hereafter a unit volume is considered)

$$
\begin{equation*}
d E=d Q+\sigma_{i j} d \varepsilon_{i j} \tag{2.109}
\end{equation*}
$$

To derive the constitutive relations of thermoelasticity theory, we transform (2.109) for the reversible processes (2.66) to the thermodynamic relation:

$$
\begin{equation*}
d E=\sigma_{i j} d \varepsilon_{i j}+T d S \tag{2.110}
\end{equation*}
$$

equating the total differential of the internal energy $\left(2.62_{2}\right)$ and the sum of the increment of the deformation work and the amount of the heat introduced into the considered volume; here $T$ is the absolute temperature and $S$ is the entropy density (2.68). We see that $E$ is a state function of $\varepsilon$ and $S$ for the reversible adiabatic (isentropic, $S=$ const.) processes, and according to the ordinary rules of differentiation, we get

$$
\begin{equation*}
\sigma_{i j}=\left.\frac{\partial E}{\partial \varepsilon_{i j}}\right|_{S}, \quad T=\left.\frac{\partial E}{\partial S}\right|_{\varepsilon} \tag{2.111}
\end{equation*}
$$

On the other hand, if the process is isothermal ( $T=$ const.), the dependence of the stresses $\boldsymbol{\sigma}$ on the strains $\boldsymbol{\varepsilon}$ is found by introducing the Helmholtz free-energy function:

$$
\begin{equation*}
F=E-T S, \quad d F=\sigma_{i j} d \varepsilon_{i j}-S d T \tag{2.112}
\end{equation*}
$$

with the strains and temperature as its independent variables. The value $-T S$ is the irreversible heat energy due to entropy as related to temperature, with
the negative sign indicating that the compressive reaction results from thermal expansion in a restrained body. Then, from Eqs. (2.110) and (2.112), we get

$$
\begin{equation*}
\sigma_{i j}=\left.\frac{\partial F}{\partial \varepsilon_{i j}}\right|_{T}, \quad S=-\left.\frac{\partial F}{\partial T}\right|_{\varepsilon} \tag{2.113}
\end{equation*}
$$

Thus, the strain energy density $w$ can be identified with the internal energy $E$ in an isentropic process ( $S \equiv$ const.) and the free energy $F$ in an isothermal process ( $T \equiv$ const.). The temperature is determined by Eq. (4.32) when using the potential $E$ and the entropy is determined by Eq. (2.113) when using the potential $F$.

For small deformations and small temperature changes $\theta=T-T^{0}\left(\theta / T_{0} \ll\right.$ 1), $F(\varepsilon, T)$ can be expanded in a power series of its arguments in the neighborhood of the virgin state $(\varepsilon=\mathbf{0}, \theta=0)$ :

$$
\begin{equation*}
F(\varepsilon, T)=\frac{1}{2} L_{i j k l} \varepsilon_{i j} \varepsilon_{k l}+\alpha_{i j}^{T} \varepsilon_{i j} \theta-\frac{C_{\varepsilon}}{2 T_{0}} \theta^{2} \tag{2.114}
\end{equation*}
$$

where one introduces the notations

$$
\begin{equation*}
L_{i j k l}=\frac{\partial^{2} F\left(\mathbf{0}, T_{0}\right)}{\partial \varepsilon_{i j} \partial \varepsilon_{k l}}, \quad \alpha_{i j}^{T}=\frac{\partial^{2} F\left(\mathbf{0}, T_{0}\right)}{\partial \varepsilon_{i j} \partial T}, \quad \frac{C_{\epsilon}}{T_{0}}=-\frac{\partial^{2} F\left(\mathbf{0}, T_{0}\right)}{\partial T^{2}} \tag{2.115}
\end{equation*}
$$

and $C_{\varepsilon}$ denotes the specific heat at constant strain, which can be also determined as

$$
\begin{equation*}
C_{\epsilon}=\left.\frac{d Q}{d T}\right|_{\epsilon}=\left.\frac{\partial E}{\partial T}\right|_{\epsilon}=\left.T \frac{\partial S}{\partial T}\right|_{\epsilon} \tag{2.116}
\end{equation*}
$$

The negative sign for the last term on the right-hand side of Eq. (2.114) indicates that a temperature rise leads to a compressive reaction on the restricted body. This term signifies the thermal energy due to temperature while the second term $\boldsymbol{\alpha}^{T} \boldsymbol{\varepsilon} \theta$ describes the coupling effect between temperature and mechanical deformation. The difference between the isentropic moduli $\tilde{\mathbf{L}}$ and isothermic moduli L:

$$
\tilde{\mathbf{L}}-\mathbf{L}=\frac{T_{0}}{\rho C_{\epsilon}} \boldsymbol{\alpha}^{T} \otimes \boldsymbol{\alpha}^{T}, \quad \tilde{\mathbf{L}}=\left.\frac{\partial^{2} E}{\partial \boldsymbol{\varepsilon} \otimes \partial \boldsymbol{\varepsilon}}\right|_{S}, \quad \mathbf{L}=\left.\frac{\partial^{2} F}{\partial \boldsymbol{\varepsilon} \otimes \partial \boldsymbol{\varepsilon}}\right|_{T}
$$

is of as order of $1 \%$ or less for metals and ceramics.
To determine the entropy as a function of $\varepsilon$ and $T$, consider the total differential of the function $S(\varepsilon, T)$ :

$$
\begin{equation*}
d S=\left.\frac{\partial S}{\partial \varepsilon}\right|_{T}: d \varepsilon+\left.\frac{\partial S}{\partial T}\right|_{\epsilon} d T \tag{2.117}
\end{equation*}
$$

Taking Eqs. (2.113) and (2.116) into account and that

$$
\begin{equation*}
\left.\frac{\partial \boldsymbol{\sigma}}{\partial T}\right|_{\varepsilon}=\left.\frac{\partial^{2} F}{\partial T \partial \varepsilon}\right|_{\varepsilon}=\boldsymbol{\alpha}^{T} \tag{2.118}
\end{equation*}
$$

equation (2.117) becomes

$$
\begin{equation*}
d S=-\boldsymbol{\alpha}^{T}: d \varepsilon+\frac{C_{\varepsilon}}{T} d T \tag{2.119}
\end{equation*}
$$

which after integration under the virgin state conditions $\boldsymbol{\varepsilon}=\mathbf{0}$ and $T=T_{0}$ gives, for small temperature change $\left(\theta / T_{0} \ll 1\right)$,

$$
\begin{equation*}
S=-\boldsymbol{\alpha}^{T}: \varepsilon+\frac{C_{\varepsilon}}{T_{0}} \theta \tag{2.120}
\end{equation*}
$$

The Gibbs thermodynamic potential per unit volume is defined for the case where the stress and temperature are chosen as the independent state variables

$$
\begin{equation*}
G_{e}=F-T S, \quad d G_{e}=-\varepsilon_{i j} d \sigma_{i j}-S d T \tag{2.121}
\end{equation*}
$$

Since $d G_{e}$ is a total differential (as $d E$ and $d F$ ), it follows that

$$
\begin{equation*}
\left.\frac{\partial G_{e}}{\partial T}\right|_{\sigma}=-S,\left.\quad \frac{\partial G_{e}}{\partial \sigma_{i j}}\right|_{T}=-\varepsilon_{i j} \tag{2.122}
\end{equation*}
$$

Thus, $-G_{e}$ can be identified with the complementary energy function $W^{c}$, which has the property that $\partial W^{c} / \partial \sigma_{i j}=\varepsilon_{i j}$. In the neighborhood of the virgin state $\left(\theta / T_{0} \ll 1, \quad \mathbf{M}::(\boldsymbol{\sigma} \otimes \boldsymbol{\sigma}) \ll 1 ; \mathbf{M}\right.$ is the compliance tensor) we have a power series expansion

$$
\begin{equation*}
G_{e}(\boldsymbol{\sigma}, T)=-\frac{1}{2} M_{i j k l} \sigma_{i j} \sigma_{k l}-\beta_{i j}^{T} \sigma_{i j} \theta+\frac{1}{2} C_{\sigma} \frac{\theta^{2}}{T_{0}} \tag{2.123}
\end{equation*}
$$

depending on the coefficient of thermal expansion $\boldsymbol{\beta}^{T}$ and the specific heat at constant stress $C_{\sigma}$

$$
\begin{equation*}
\left.\boldsymbol{\beta}^{T} \equiv \frac{\partial \boldsymbol{\varepsilon}}{\partial T}\right|_{\sigma}=-\mathbf{M} \boldsymbol{\alpha}^{T}, \quad C_{\sigma}=\left.\frac{d Q}{d T}\right|_{\sigma} \tag{2.124}
\end{equation*}
$$

where $C_{\sigma}$ is related to $C_{\epsilon}$ by the equation

$$
\begin{equation*}
C_{\sigma}=C_{\varepsilon}+L_{i j k l} \boldsymbol{\beta}_{i j}^{T} \beta_{k l}^{T} T_{0} \tag{2.125}
\end{equation*}
$$

Analogously to the tensor $\mathbf{L}$ (2.90), the complience tensor $\mathbf{M}$ is also fullsymmetric.

Substitution of the representation (2.114) into Eq. (2.113 $)_{1}$ ) yields the general Duhamel-Newmann form of Hook's law for an anisotropic body in the form of invertible relations:

$$
\begin{equation*}
\sigma=\mathbf{L} \varepsilon+\alpha, \quad \varepsilon=\mathrm{M} \sigma+\beta \tag{2.126}
\end{equation*}
$$

where $\boldsymbol{\alpha} \equiv \boldsymbol{\alpha}^{T} \theta, \boldsymbol{\beta}^{\top}=\boldsymbol{\beta} / \theta$ is the coefficient of thermal expansion related to $\boldsymbol{\alpha}$ by $\boldsymbol{\alpha}=-\mathbf{L} \boldsymbol{\beta}, \alpha_{i j}=-L_{i j k l} \beta_{k l}$; see Eq. (2.124). $\boldsymbol{\beta}(\mathbf{x})$ and $\boldsymbol{\alpha}(\mathbf{x}) \equiv-\mathbf{L}(\mathbf{x}) \boldsymbol{\beta}(\mathbf{x})$ are second-order tensors of local eigenstrains and eigenstresses, respectively (frequently called transformation fields) which may arise by thermal expansion, phase transformation, twinning, and other changes of shape or volume of the material. Substitution of the constitutive equation $\left(2.126_{1}\right)$ into the motion equation (2.60) at small deformations (2.92) leads to the Navier equation:

$$
\begin{equation*}
\nabla(\mathbf{L} \nabla \mathbf{u})+\rho \mathbf{b}=\rho \ddot{\mathbf{u}}-\boldsymbol{\alpha}^{T} \nabla \theta, \quad \frac{\partial}{\partial x_{j}}\left(L_{i j k l} \frac{\partial u_{k}}{\partial x_{l}}\right)+\rho b_{i}=\rho \frac{\partial^{2} u_{j}}{\partial t^{2}}-\alpha_{i j}^{T} \frac{\partial \theta}{x_{j}} \tag{2.127}
\end{equation*}
$$

which should be considered simultaneously with the generalized heat condition equation:

$$
\begin{equation*}
\nabla(\boldsymbol{\lambda} \nabla \theta)-C_{\varepsilon} \dot{\theta}+T_{0} \boldsymbol{\alpha}^{T}: \dot{\boldsymbol{\varepsilon}}=-h, \quad \frac{\partial}{\partial x_{i}}\left(\lambda_{i j} \frac{\partial}{\partial x_{j}} \theta\right)-C_{\varepsilon} \frac{\partial \theta}{\partial t}+T_{0} \alpha_{i j}^{T} \frac{\partial \varepsilon_{i j}}{\partial t}=-h \tag{2.128}
\end{equation*}
$$

where $h$ is the heat source function. The coefficient $\boldsymbol{\alpha}^{T}$ couples the equations of the dynamic thermoelastic problem (2.127) and (2.128). Dropping the term $T_{0} \boldsymbol{\alpha}^{T}: \dot{\varepsilon}$ in the left-hand side of Eq. $\left(2.128_{1}\right)$, we obtain the uncoupled equations of dynamic thermoelasticity. Further neglecting the inertial term $\rho \ddot{\mathbf{u}}$ in Eq. (2.127) leads to a system of equations of the quasistatic thermoelastic problem.

The system (2.127) and (2.128) should be accompanied by appropriate boundary and initial conditions. The initial conditions specify the temperature, displacements, and velocities at all points $\mathbf{x} \in \mathcal{E}$ within $\mathcal{E}: \theta(\mathbf{x}, 0)=\theta_{0}(\mathbf{x})$, $\mathbf{u}(\mathbf{x}, 0)=\mathbf{u}_{0}(\mathbf{x}), \dot{\mathbf{u}}(\mathbf{x}, 0)=\mathbf{v}_{0}(\mathbf{x})$ with the prescribed functions $\theta_{0}, \mathbf{u}_{0}, \mathbf{v}_{0}$ on $\mathcal{E}$. The most widely used boundary condition for the temperature function $\theta$ is expressed by

$$
\begin{equation*}
\boldsymbol{\lambda}:(\nabla \theta(\mathbf{x}, t) \otimes \mathbf{n})+\alpha_{\partial \mathcal{E}}\left[\theta(\mathbf{x}, t)-\theta_{\partial \mathcal{E}}(\mathbf{x}, t)\right]=q_{0}(\mathbf{x}, t) \tag{2.129}
\end{equation*}
$$

with $\theta_{\partial \mathcal{E}}, q_{0}$ as known functions of the time and point $\mathbf{x} \in \partial \mathcal{E}$. If $q_{0} \equiv 0$, Eq. (2.129) describes heat exchange of the third kind called Newton's law. The conditions $\alpha_{\partial \mathcal{E}}=\infty$ and $q_{0} \equiv 0$ reduce Eq. (2.129) to the boundary conditions of the first kind. We obtain the second kind of boundary conditions if $\alpha_{\partial \mathcal{E}} \equiv 0$. The mechanical boundary conditions (2.96) and (2.97) should be added to the thermal boundary conditions (2.129). In so doing, the traction $\mathbf{t}(\mathbf{x})=\boldsymbol{\sigma}(\mathbf{x}) \mathbf{n}(\mathbf{x})$ acting on any surface with the normal $\mathbf{n}(\mathbf{x})$ through the point $\mathbf{x}$ can be represented in terms of displacements:

$$
\begin{equation*}
\mathbf{t}(\mathbf{x})=\hat{\mathbf{t}}(\mathbf{n}, \nabla) \mathbf{u}(\mathbf{x})+\boldsymbol{\alpha} \mathbf{n}, \quad \hat{t}_{i k}(\mathbf{n}, \nabla) \equiv L_{i j k l} n_{j}(\mathbf{x}) \partial / \partial x_{l}, \tag{2.130}
\end{equation*}
$$

where $\hat{t}_{i k}(\mathbf{n}, \nabla)$ is the so-called "stress operator."
We will consider the uncoupled quasistatic thermoelasticity theory $[\rho \ddot{\mathbf{u}} \equiv \mathbf{0}$ in Eq. (2.127) and $T_{0} \boldsymbol{\alpha}^{T}: \dot{\boldsymbol{\varepsilon}} \equiv \mathbf{0}$ in Eq. (2.128)], where the temperature field is determined by Eq. (2.128) with no influence of the latent heat due to the change of strain. It takes place according to so-called body force analogy [197] asserting that $\{\mathbf{u}, \boldsymbol{\varepsilon}, \boldsymbol{\sigma}\}$ is a solution of the mixed problem of thermoelastostatics corresponding to the external loading conditions ( $\left.\mathbf{b}, \mathbf{t}^{\partial \mathcal{E}_{\sigma}}, \mathbf{u}^{\partial \mathcal{E}_{\mathbf{u}}}, \theta\right)$ on $\mathcal{E} \cup \partial \mathcal{E}_{\sigma} \cup$ $\partial \mathcal{E}_{\mathbf{u}}$ if and only if $\{\mathbf{u}, \boldsymbol{\varepsilon}, \boldsymbol{\sigma}-\boldsymbol{\alpha}\}$ is a solution of the mixed problem of elastostatic corresponding to the external loading ( $\left.\mathbf{b}+1 / \rho \nabla \boldsymbol{\alpha}, \mathbf{t}^{\partial \mathcal{E}_{\sigma}}-\boldsymbol{\alpha} \cdot \mathbf{n}, \mathbf{u}^{\partial \mathcal{E}_{\mathbf{u}}}\right)$ on $\mathcal{E} \cup$ $\partial \mathcal{E}_{\sigma} \cup \partial \mathcal{E}_{\mathbf{u}}$.

To present the extremum principles of linear thermoelastostatic, determine the total strain energy $W(\varepsilon)$ and total stress energy $W^{c}(\boldsymbol{\sigma})$ :

$$
\begin{align*}
W(\boldsymbol{\varepsilon}) & =\int_{\mathcal{E}} w(\boldsymbol{\varepsilon}) d \mathbf{x}, \quad w(\boldsymbol{\varepsilon})=\frac{1}{2}(\boldsymbol{\varepsilon}-\boldsymbol{\beta}): \mathbf{L}:(\boldsymbol{\varepsilon}-\boldsymbol{\beta}) \\
W^{c}(\boldsymbol{\sigma}) & =\int_{\mathcal{E}} w^{c}(\boldsymbol{\sigma}) d \mathbf{x}, \quad w^{c}(\boldsymbol{\sigma})=\frac{1}{2}(\boldsymbol{\sigma}-\boldsymbol{\alpha}): \mathbf{M}:(\boldsymbol{\sigma}-\boldsymbol{\alpha}) \tag{2.131}
\end{align*}
$$

yielding Eqs. (2.102) at $\boldsymbol{\alpha}=\boldsymbol{\beta} \equiv \mathbf{0} . w(\boldsymbol{\varepsilon})$ and $w^{c}(\boldsymbol{\sigma})$ are potential functions for determining the stress and strain according to the equations

$$
\begin{equation*}
\sigma_{i j}=\frac{\partial w(\boldsymbol{\varepsilon})}{\partial \varepsilon_{i j}}, \quad \varepsilon_{i j}=\frac{\partial w^{c}(\boldsymbol{\sigma})}{\partial \sigma_{i j}}, \tag{2.132}
\end{equation*}
$$

respectively, which are valid for both elasticity and thermoelasticity; in linear theory $w(\boldsymbol{\varepsilon})$ and $w^{c}(\boldsymbol{\sigma})$ are equal: $w=w^{c}$. Positiveness of $w$ and $w^{c}($ at $\boldsymbol{\varepsilon}, \boldsymbol{\sigma} \neq \mathbf{0})$ leads to the positive definiteness of $\mathbf{L}$ and $\mathbf{M}$, respectively (i.e. $\varepsilon: \mathbf{L}: \varepsilon>0$ for $\forall \varepsilon \neq \mathbf{0}$ ).

The classical extremum principles (2.106) and (2.108) for pure mechanical loading can be generalized for the thermostatic case in the following manner. We determine the potential $\Pi$ and complementary $\Pi^{c}$ energies:

$$
\begin{align*}
\Pi\left(\tilde{s}^{k}\right) & =W(\tilde{\boldsymbol{\varepsilon}})-\int_{\mathcal{E}} \rho \mathbf{b} \cdot \tilde{\mathbf{u}} d \mathbf{x}-\int_{\partial \mathcal{E}_{\sigma}} \mathbf{t}^{\partial \mathcal{E}_{\sigma}} \cdot \tilde{\mathbf{u}} d \mathbf{s} \\
\Pi^{c}\left(\tilde{s}^{s}\right) & =W^{c}(\tilde{\boldsymbol{\sigma}})-\int_{\partial \mathcal{E}_{\mathbf{u}}} \tilde{\mathbf{t}}_{\mathbf{n}} \cdot \mathbf{u}^{\partial \mathcal{E}_{\mathbf{u}}} d \mathbf{s}, \quad \tilde{\mathbf{t}}_{\mathbf{n}} \equiv \tilde{\boldsymbol{\sigma}} \cdot \mathbf{n}^{\partial \mathcal{E}} \tag{2.133}
\end{align*}
$$

defined on the sets of kinematically $\tilde{s}^{k}=\{\tilde{\mathbf{u}}, \tilde{\varepsilon}, \tilde{\boldsymbol{\sigma}}\} \in \mathcal{A}^{k}(\tilde{s})$ (2.100) and statically $\tilde{s}^{s}=\{\tilde{\mathbf{u}}, \tilde{\varepsilon}, \tilde{\boldsymbol{\sigma}}\} \in \mathcal{A}^{s}(\tilde{s})$ (2.101) admissible states, respectively. In so doing one assumes that Eqs. (2.94) is replaced by Eq. (2.1261) to include the $\tilde{\boldsymbol{\sigma}}$ and $\tilde{\boldsymbol{\varepsilon}}$ generated by $\tilde{\mathbf{u}} \in \mathcal{A}^{k}(\tilde{\mathbf{u}})$ and by $\tilde{\boldsymbol{\sigma}} \in \mathcal{A}^{s}(\tilde{\boldsymbol{\sigma}})$, respectively. Then the actual state $s=\{\mathbf{u}, \boldsymbol{\varepsilon}, \boldsymbol{\sigma}\}$ renders the potential energy $\Pi$ and complementary energy $\Pi^{c}$ an absolute minimum

$$
\begin{equation*}
\Pi(s) \leq \Pi\left(\tilde{s}^{k}\right), \quad \Pi^{c}(s) \leq \Pi^{c}\left(\tilde{s}^{s}\right) \tag{2.134}
\end{equation*}
$$

### 2.4.2 Electroelastic Equations

Another example of the theory of coupled fields is associated with electroelasticity and deals with phenomena caused by interactions between the elastic, electric, and thermal fields. The relations between the mechanical, electrical, and thermal properties can be demonstrated from equilibrium thermodynamics described by the potential Gibbs function:

$$
\begin{equation*}
G_{e}=E-\mathbf{E} \cdot \mathbf{D}-S T, \tag{2.135}
\end{equation*}
$$

whose differential form

$$
\begin{equation*}
d G_{e}=\sigma_{i j} d \varepsilon_{i j}-D_{i} d E_{i}-S d T \tag{2.136}
\end{equation*}
$$

yields the relations

$$
\begin{equation*}
\sigma_{i j}=\left.\frac{\partial G_{e}}{\partial \varepsilon_{i j}}\right|_{\mathbf{E}, T}, \quad D_{m}=-\left.\frac{\partial G_{e}}{\partial E_{m}}\right|_{\varepsilon, T}, \quad S=-\left.\frac{\partial G_{e}}{\partial T}\right|_{\varepsilon, \mathbf{E}} \tag{2.137}
\end{equation*}
$$

Differentiating Eqs. (2.137) gives

$$
\begin{equation*}
\left.\frac{\partial \sigma_{i j}}{\partial E_{m}}\right|_{\varepsilon, T}=-\left.\frac{\partial D_{m}}{\partial \varepsilon}\right|_{\mathbf{E}, T},\left.\quad \frac{\partial \sigma_{i j}}{\partial T}\right|_{\varepsilon, \mathbf{E}}=-\left.\frac{\partial S}{\partial \varepsilon_{i j}}\right|_{\mathbf{E}, T},\left.\quad \frac{\partial D_{m}}{\partial T}\right|_{\varepsilon, \mathbf{E}}=\left.\frac{\partial S}{\partial E_{m}}\right|_{\varepsilon, T} \tag{2.138}
\end{equation*}
$$

It was assumed that the strains $\varepsilon$, the electric field intensity $\mathbf{E}$, and the temperature $T=T_{0}+\theta$ are considered the independent variables, and the dependent variables will be the stresses $\sigma$, the vector of induction $\mathbf{D}$, and the entropy $S$. $G_{e}$ is a state function of $\boldsymbol{\varepsilon}, \mathbf{E}$, and $T$, and the ordinary rules of differentiation yield the explicite representation of $d \boldsymbol{\sigma}, d \mathbf{D}$, and $d S$ as the functions of $d \boldsymbol{\varepsilon}, d \mathbf{E}$, and $d T$ :

$$
\begin{align*}
d \sigma_{i j} & =\left.\frac{\partial \sigma_{i j}}{\varepsilon_{k l}}\right|_{\mathbf{E}, T} d \varepsilon_{k l}+\left.\frac{\partial \sigma_{i j}}{E_{m}}\right|_{\varepsilon, T} d E_{m}+\left.\frac{\partial \sigma_{i j}}{\partial T}\right|_{\mathbf{E}, \varepsilon} d T \\
d D_{m} & =\left.\frac{\partial D_{m}}{\varepsilon_{k l}}\right|_{\mathbf{E}, T} d \varepsilon_{k l}+\left.\frac{\partial D_{m}}{E_{k}}\right|_{\varepsilon, T} d E_{k}+\left.\frac{\partial D_{m}}{\partial T}\right|_{\varepsilon, \mathbf{E}} d T \\
d S & =\left.\frac{\partial S}{\varepsilon_{k l}}\right|_{\mathbf{E}, T} d \varepsilon_{k l}+\left.\frac{\partial S}{E_{m}}\right|_{\varepsilon, T} d E_{m}+\left.\frac{\partial S}{\partial T}\right|_{\varepsilon \mathbf{E}} d T \tag{2.139}
\end{align*}
$$

For small variations of $\boldsymbol{\varepsilon}, \mathbf{E}$, and $T$ in the neighborhood of virgin state $(\varepsilon, \mathbf{E}, \theta=$ $\mathbf{0}$ ), the partial derivatives can be considered as constants and Eqs. (2.139) becomes integrable. This allows us to obtain the constitutive relations of a deformable piezoelectric medium (see, e.g., [747], [861]):

$$
\begin{align*}
\sigma_{i j} & =L_{i j k l} \varepsilon_{k l}+e_{k i j} E_{k}+\alpha_{i j}^{T} \theta, \\
D_{i} & =e_{i j k} \varepsilon_{j k}-k_{i j} E_{j}+p_{i} \theta, \quad d S=\alpha_{i j} \varepsilon_{i j}+p_{i} E_{i}+c_{\varepsilon} \theta / T_{0} \tag{2.140}
\end{align*}
$$

where the coefficients of the system (2.140) denote the particular derivatives:

$$
\begin{align*}
L_{i j k l} & =\left.\frac{\partial \sigma_{i j}}{\partial \varepsilon_{k l}}\right|_{\mathbf{E}, T}, \quad e_{i j k}=\left.\frac{\partial \sigma_{i j}}{\partial E_{k}}\right|_{\varepsilon, T} \\
p_{i} & =\left.\frac{\partial D_{i}}{\partial T}\right|_{\varepsilon, \mathbf{E}}, \quad k_{i j}=\left.\frac{\partial D_{i}}{\partial E_{j}}\right|_{\varepsilon, T} \\
\alpha_{i j}^{T} & =\left.\frac{\partial \sigma_{i j}}{\partial T}\right|_{\varepsilon, \mathbf{E}}, \quad \frac{c_{\varepsilon}}{T_{0}}=\left.\frac{\partial S}{\partial T}\right|_{\varepsilon, \mathbf{E}} \tag{2.141}
\end{align*}
$$

The constitutive equations $\left(2.140_{1}\right)$ and $\left(2.140_{2}\right)$ can be inverted

$$
\begin{align*}
\varepsilon_{i j} & =M_{i j k l} \sigma_{k l}+d_{k i j} D_{k}+\beta_{i j}^{T} \theta  \tag{2.142}\\
E_{i} & =d_{i j k} \sigma_{j k}-b_{i j} D_{j}+q_{i} \theta \tag{2.143}
\end{align*}
$$

The coefficients of Eqs. (2.140) and (2.142), (2.143) are denoted as follows: $\mathbf{L}$ and $\mathbf{M}$ are the elastic and compliance tensors, $\mathbf{k}$ and $\mathbf{b}$ are the tensors of dielectric permeability and impermeability, e and $\mathbf{d}$ are the piezoelectric moduli, $\boldsymbol{\alpha}^{T}$ and $\boldsymbol{\beta}^{T}$ are the coefficients of thermoelastic stress and expansion, $\mathbf{p}$ and $\mathbf{q}$ are the pyroelectric coefficients; and $c_{\varepsilon}$ is a specific heat per unit volume at constant strain. To obtain a symmetric matrix of coefficients we replace the electric field $\mathbf{E}$ by $\mathbf{- E}$. Hereinafter in Subsection 2.4.3, the Latin indexes range from 1 to 3,
and the Greek ones range from 1 to 4 . We will assume that the electric and elastic fields are fully coupled, but temperature enters the problem only as a parameter through the constitutive equation. The tensor coefficients in the above equations are related as $e_{k i j}=d_{k m n} L_{m n i j}, d_{k i j}=e_{k m n} M_{m n i j}, M_{i j m n} L_{m n k l}=I_{i j k l}$. We will use the matrix notation instead of the tensor one, as it is accepted in the theory of elasticity. For notational convenience the elastic and electric variable will be treated on equal footing, and with this in mind we recast the local linear constitutive relations of thermoelectroelasticity for this material in the notation introduced in [38]. For this purpose, we introduce the matrices of generalized stresses $\boldsymbol{\Sigma}$ and strains $\mathcal{E}(i, j=1,2,3)$ :

$$
\begin{align*}
& \boldsymbol{\Sigma}=\binom{\boldsymbol{\sigma}}{\mathbf{D}}, \quad \Sigma_{i j}=\sigma_{i j}, \quad \Sigma_{i 4}=D_{i}  \tag{2.144}\\
& \mathcal{E}=\binom{\boldsymbol{\varepsilon}}{\mathbf{E}}, \quad \mathcal{E}_{i j}=\varepsilon_{i j}, \quad \mathcal{E}_{4 i}=E_{i} \tag{2.145}
\end{align*}
$$

which are interrelated through the matrices of generalized elastic coefficients $\mathbb{L}_{\alpha \beta \gamma \delta}$, compliance $\mathbb{M}_{\alpha \beta \gamma \delta}$, and generalized coefficients of thermal expansion $\Lambda_{\alpha \beta}$ :

$$
\begin{equation*}
\boldsymbol{\Sigma}=\mathbb{L}(\mathcal{E}-\boldsymbol{\Lambda}), \quad \mathcal{E}=\mathbb{M} \boldsymbol{\Sigma}+\boldsymbol{\Lambda} \tag{2.146}
\end{equation*}
$$

where the generalized coefficients can be presented in matrix form:

$$
\begin{equation*}
\mathbb{L}=\binom{\mathbf{L} \mathbf{e}^{\top}}{\mathbf{e}-\mathbf{k}}, \quad \mathbb{M}=\binom{\mathbf{M} \mathbf{d}^{\top}}{\mathbf{d}-\mathbf{b}}, \quad \boldsymbol{\Lambda}=\binom{\boldsymbol{\beta}^{T} \theta}{\mathbf{q} \theta} \tag{2.147}
\end{equation*}
$$

as well as in component form:

$$
\begin{align*}
\mathbb{L}_{i j k l} & =L_{i j k l}, \mathbb{L}_{4 i k l}=\mathbb{L}_{i 4 k l}=e_{i k l},  \tag{2.148}\\
\mathbb{L}_{i j 4 l} & =\mathbb{L}_{i j l 4}=e_{l i j}, \mathbb{L}_{44 k l}=\mathbb{L}_{k l 44}=-k_{k l},  \tag{2.149}\\
\mathbb{M}_{i j k l} & =M_{i j k l}, \mathbb{M}_{4 i k l}=\mathbb{M}_{i 4 k l}=d_{i k l},  \tag{2.150}\\
\mathbb{M}_{i j 4 l} & =\mathbb{M}_{i j l 4}=d_{l i j}, \mathbb{M}_{44 k l}=\mathbb{M}_{k l 44}=-b_{k l},  \tag{2.151}\\
\Lambda_{i j} & =\beta_{i j} \theta, \Lambda_{4 i}=\Lambda_{i 4}=q_{i} \theta \tag{2.152}
\end{align*}
$$

The generalized static equation and the Cauchy conditions of small generalized deformations are expressed in the form:

$$
\begin{align*}
\nabla_{\alpha} \Sigma_{\alpha \beta} & =f_{\beta}, \quad \nabla_{4} \equiv 0  \tag{2.153}\\
\mathcal{E}_{\alpha \beta} & =\left(\nabla_{\alpha} U_{\beta}+\nabla_{\beta} U_{\alpha}\right) / 2 \tag{2.154}
\end{align*}
$$

where $U=\left(u_{1}, u_{2}, u_{3},-\phi\right)^{\top}, \phi$ is the electrostatic potential, and $f_{\beta}$ is a density of generalized body forces. Equations (2.153) and (2.154) should be complemented by the boundary conditions of the first or the second type. Except for notations, these equations coincide with the equations of linear thermoelasticity (2.126). Because of this the theory of piezoelectric composite materials (PCM) retraces at a particular instant the path of development of the theory of microinhomogeneous elastic media, exhibiting substantial progress. In light of the analogy mentioned in our brief survey we will not consider in detail the PCM; one may refer instead to the appropriate scheme of thermoelastic composites.

### 2.4.3 Matrix Representation of Some Symmetric Tensors

Many material (e.g., elastic moduli (2.90)) and field (e.g., the stresses (2.50)) tensors have internal symmetry with respect to pairs of indices. Although all calculations with these tensors can be done by common methods of tensor analysis (see Section 2.2), it is sometimes convenient to introduce a contracted notation significantly simplifying the calculations. It is possible to form six different pairs $i j$ for permuted indices $i, j(i, j=1,2,3)$. Then the pair of these Latin indices ij can be substituted by a single Greek index $\alpha$ ranging from 1 to 6 according to the Voight notation:

$$
\begin{align*}
& \text { tensor notation } i j=11 \quad 2233233213311221 \\
& \text { matrix notation } \alpha=\begin{array}{llllll}
1 & 2 & 3 & 4 & 5 & 6
\end{array} \text {, } \tag{2.155}
\end{align*}
$$

which can be presented in a formula form (no sum on $i, j$ ):

$$
\begin{equation*}
\alpha=i \delta_{i j}+(9-i-j)\left(1-\delta_{i j}\right) . \tag{2.156}
\end{equation*}
$$

To avoid confusion, it should be mentioned that the prescribed reduction rule $i j \leftrightarrow \alpha$ between the Latin and Greek indices is also used for Latin indices $i j \leftrightarrow k$ $(i, j=1,2,3, k=1, \ldots, 6)$ where the correspondence between the pair indices and one index is provided by the scheme (2.155). We will also use this traditional form of an accordance.

We accept the convention that for the tensors with the prescribed aforementioned symmetry, Greek indices $\alpha, \beta, \gamma$ range from 1 to 6 and are connected with the pairs of Latin indices $i j, k l$ and others by relation (2.155). In such a case, the tensor representations of the thermoelastic constitutive equations (2.126) can be presented in matrix notations:

$$
\begin{equation*}
\sigma_{\gamma}=L_{\gamma \delta} \varepsilon_{\delta}+\alpha_{\gamma}, \quad \varepsilon_{\gamma}=M_{\gamma \delta} \sigma_{\delta}+\beta_{\gamma} . \tag{2.157}
\end{equation*}
$$

It is also required that the tensor representations of widely used scalar productions $\mathbf{L}: \mathbf{M}$ and $\boldsymbol{\sigma}: \boldsymbol{\varepsilon}$ have an appropriate vector form $L_{\lambda \mu} M_{\mu_{\nu}}=\delta_{\lambda \nu}$, $\sigma_{i j} \varepsilon_{i j}=\sigma_{\lambda} \varepsilon_{\lambda}$ with the standard assumption of summation over the repeated indices. However, it is conceivable to show that if the quantities corresponding to the different tensors are formed by one and the same rule (such as e.g. $\sigma_{i j}=\sigma_{\lambda}$ ), the equalities either (2.156) or (2.157) cannot be fulfilled. The mentioned inconsistency can be avoided if the following matrix representations of tensors are used $(i, j, k, l=1,2,3 ; \lambda, \mu=1, \ldots, 6)$

$$
\begin{align*}
\varepsilon_{i j} & =\frac{\varepsilon_{\lambda}}{2-\delta_{i j}}, \quad \sigma_{i j}=\sigma_{\lambda}  \tag{2.158}\\
L_{i j k l} & =L_{\lambda \mu}, \quad M_{i j k l}=\frac{M_{\lambda \mu}}{\left(2-\delta_{i j}\right)\left(2-\delta_{k l}\right)} . \tag{2.159}
\end{align*}
$$

In so doing, the transition to Greek indices for all symmetric second-rank tensors could be done in the unique manner $\alpha_{i j}=\left(2-\delta_{i j}\right)^{-1 / 2} \alpha_{\lambda}$.

Since the components of the stress and strain tensors are functions of the orientation of the reference system, the elastic coefficients in Eqs. (2.126 $)$ (and,
analogously, in Eq. $\left(2.126_{2}\right)$ ) are also tensor functions of this orientation and they can be presented in a new coordinate system:

$$
\begin{equation*}
\mathbf{L}^{\prime}=\operatorname{gg}_{\mathbf{L}} \mathbf{g}^{\top} \mathbf{g}^{\top}, L_{i j k l}^{\prime}=q_{i m} g_{j n} g_{k o} g_{l p} L_{m n o p} \tag{2.160}
\end{equation*}
$$

with the relevant transformation for the strain tensors:

$$
\begin{equation*}
\varepsilon^{\prime}=\operatorname{gg} \varepsilon, \quad \varepsilon_{i j}^{\prime}=g_{i k} g_{j l} \varepsilon_{k l} \tag{2.161}
\end{equation*}
$$

These tensors as well as the thermal expansion and thermoelasticity coefficients are transformed using Voight reduced components as

$$
\begin{align*}
& \mathbf{L}^{\prime}=\mathbf{q}^{L} \mathbf{L}\left(\mathbf{q}^{L}\right)^{\top}, \quad \mathbf{M}^{\prime}=\mathbf{q}^{R} \mathbf{M}\left(\mathbf{q}^{R}\right)^{\top}  \tag{2.162}\\
& \boldsymbol{\sigma}^{\prime}=\mathbf{q}^{L} \boldsymbol{\sigma}, \quad \varepsilon^{\prime}=\mathbf{q}^{R} \varepsilon, \quad \boldsymbol{\alpha}^{T^{\prime}}=\mathbf{q}^{L} \boldsymbol{\alpha}^{T}, \quad \boldsymbol{\beta}^{T^{\prime}}=\mathbf{q}^{R} \boldsymbol{\beta}^{T} \tag{2.163}
\end{align*}
$$

where the fourth-order tensors $\mathbf{q}^{L}$ and $\mathbf{q}^{R}=\left[\left(\mathbf{q}^{L}\right)^{-1}\right]^{\top}$ presented in the form of $(6 \times 6)$ transformation matrices:

$$
\mathbf{q}^{L}=\left(\begin{array}{ll}
\mathbf{K}_{1} & 2 \mathbf{K}_{2}  \tag{2.164}\\
\mathbf{K}_{3} & \mathbf{K}_{4}
\end{array}\right), \quad \mathbf{q}^{R}=\left(\begin{array}{ll}
\mathbf{K}_{1} & \mathbf{K}_{2} \\
2 \mathbf{K}_{3} & \mathbf{K}_{4}
\end{array}\right)
$$

are combined by four matrices $(3 \times 3)$ :

$$
\begin{align*}
& \mathbf{K}_{1}=\left(\begin{array}{lll}
g_{11}^{2} & g_{12}^{2} & g_{13}^{2} \\
g_{21}^{2} & g_{22}^{2} & g_{23}^{2} \\
g_{31}^{2} & g_{32}^{2} & g_{33}^{2}
\end{array}\right), \quad \mathbf{K}_{2}=\left(\begin{array}{lll}
g_{12} g_{13} & g_{13} g_{11} & g_{11} g_{12} \\
g_{22} g_{23} & g_{23} g_{21} & g_{21} g_{22} \\
g_{32} g_{33} & g_{33} g_{31} & g_{31} g_{32}
\end{array}\right) \\
& \mathbf{K}_{3}=\left(\begin{array}{lll}
g_{21} g_{31} & g_{22} g_{32} & g_{23} g_{33} \\
g_{31} g_{11} & g_{32} g_{121} & g_{33} g_{13} \\
g_{11} g_{21} & g_{12} g_{22} & g_{13} g_{23}
\end{array}\right) \\
& \mathbf{K}_{4}=\left(\begin{array}{lll}
g_{22} g_{33}+g_{23} g_{32} & g_{23} g_{31}+g_{21} g_{33} & g_{21} g_{32}+g_{22} g_{31} \\
g_{32} g_{13}+g_{33} g_{12} & g_{33} g_{11}+g_{31} g_{13} & g_{31} g_{12}+g_{32} g_{11} \\
g_{12} g_{23}+g_{13} g_{22} & g_{13} g_{21}+g_{11} g_{23} & g_{11} g_{22}+g_{12} g_{21}
\end{array}\right) . \tag{2.165}
\end{align*}
$$

The elements of the matrices $\mathbf{q}^{L}$ and $\mathbf{q}^{R}$ can be represented by means of the formulae

$$
\begin{equation*}
\frac{1}{2}\left(g_{i^{\prime} k} g_{j^{\prime} l}+g_{i^{\prime} l} g_{j^{\prime} k}\right)=\frac{q_{\lambda^{\prime} \mu}^{R}}{2-\delta_{i^{\prime} j^{\prime}}}=\frac{q_{\lambda^{\prime} \mu}^{L}}{2-\delta_{k l}} \tag{2.166}
\end{equation*}
$$

where the index $\lambda^{\prime}$ corresponds to the pair $i^{\prime} j^{\prime}$, and $\mu$ to the pair $k l$.
The equations describing the piezoelectric effect (2.142) and (2.143) can be presented in the matrix form:

$$
\begin{equation*}
\varepsilon_{\gamma}=M_{\gamma \delta} \sigma_{\delta}+d_{\gamma i} E_{i}+\lambda_{\gamma}, \quad D_{i}=e_{i \gamma} \varepsilon_{\gamma}-k_{i j} E_{j}+p_{i} \theta \tag{2.167}
\end{equation*}
$$

which leads to the following recalculation rules for the coefficients $(j k \leftrightarrow \gamma)$ : $d_{i j k}=\left(2-\delta_{j k}\right)^{-1} d_{i \gamma}, \quad e_{i j k}=e_{i \gamma}$ with Latin indices ranging from 1 to 3 . The transformation formulae for quantities $d_{i \gamma}$ and $e_{i \gamma}$ in the old $\mathbf{e}_{k}$ and new $\mathbf{e}_{k}^{\prime}$ ( $k=1,2,3$ ) coordinate systems with the cosine matrix $g_{i j}(2.4)$ are

$$
\begin{equation*}
d_{i \gamma}^{\prime}=g_{i j} q_{\gamma \delta}^{R} d_{j \delta}, \quad d_{j \delta}=g_{j i}^{\top} q_{\delta \gamma}^{L} d_{i \gamma}^{\prime}, \quad e_{i \gamma}^{\prime}=g_{i j} q_{\gamma \delta}^{L} e_{j \delta}, \quad e_{j \delta}=g_{j i}^{\top} q_{\delta \gamma}^{R} e_{i \gamma}^{\prime} \tag{2.168}
\end{equation*}
$$

The recalculation rules and the transformation formulae can be obtained by the same method for other coefficients characterizing piezoelectric properties of crystals (for details see [1015]).

### 2.5 Symmetry of Elastic Properties

Under an orthogonal transformation (2.6) the elastic stiffnesses $L_{i j k l}^{\prime}$ referred to the $\mathbf{e}_{i}^{\prime}$ coordinate system are described by Eq. (2.160). When $\mathbf{L}^{\prime}=\mathbf{L}$, i.e.,

$$
\begin{equation*}
L_{i j k l}=g_{i p} g_{j q} g_{k r} g_{s t} L_{p q r t} \tag{2.169}
\end{equation*}
$$

the material is said to possesses a symmetry with respect to $\mathbf{g}$, which is called a symmetry transformation. If $\mathbf{g}$ is a symmetry transformation, $\mathbf{g}^{-1}=\mathbf{g}^{\top}$ is also a symmetry transformation, and if $\mathbf{g}$ and $\mathbf{q}$ are two symmetry transformations then gq is also a symmetry transformation. The set of all symmetry transformations at $\mathbf{x} \in \mathcal{E}$ forms a symmetry group $S_{\mathbf{x}}$ with the unit transformation $\boldsymbol{\delta}$. The group $S_{\mathbf{x}}$ always contains the two-element subgroup $\{\boldsymbol{\delta},-\boldsymbol{\delta}\}$. We describe by $\mathbf{g}(\omega \mathbf{n})$ (A.1.2) a right-handed rotation by the angle $\omega, 0<\omega<\pi$, about an axis oriented in the direction of unit vector $\mathbf{n}$. The transformation $\mathcal{R}(\mathbf{n})=-\mathbf{g}(\pi \mathbf{n})$ is called a reflection in the plane $P(\mathbf{n})$ with the normal unit vector $\mathbf{n}$. A unit vector $\mathbf{n}$ is called an axis of symmetry at $\mathbf{x}$ if $\mathbf{g n}=\mathbf{n}$ for some $\mathbf{g} \in S_{\mathbf{x}}$, with $\mathrm{g} \neq \boldsymbol{\delta}$. A plane $P\left(\mathbf{n}_{1}, \mathbf{n}_{2}\right)$, spanned by two mutually orthogonal unit vectors $\mathbf{n}_{1}$ and $\mathbf{n}_{2}$, is called a plane of symmetry at $\mathbf{x}$ if $\mathbf{g} \mathbf{n}_{1}=\mathbf{n}_{1}$ and $\mathbf{g \mathbf { n } _ { 2 }}=\mathbf{n}_{2}$, for some $\mathbf{g} \in S_{\mathbf{x}}$, with $\mathbf{g} \neq \boldsymbol{\delta}$. The condition symmetry (2.169) introduces a restriction on the elastic moduli $\mathbf{L}$. A triclinic material has a minimum symmetry group $S_{\mathbf{x}}$ formed by the two-element group $\{\boldsymbol{\delta},-\boldsymbol{\delta}\}$. According to Eq. (2.90), the number of independent elastic coefficients for the general anisotropic linearly elastic triclinic materials is reduced from $3^{4}=81$ to 21 , which is the number of terms on the main diagonal of the matrix $\mathbf{L}$ and the five subdiagonals above it: $6+5+4+3+2+1=21$. This number is reduced when the symmetry of the crystal class of medium is accounted for. A material is called monoclinic if for any $\mathbf{x} \in \mathcal{E}$ the symmetry grope $S_{\mathbf{x}}$ is formed by the transformations $\pm \boldsymbol{\delta}, \pm \mathbf{g}(\pi, \mathbf{n})$ for any $\mathbf{x} \in \mathcal{E}$. An orthotropic has the symmetry group $S_{\mathbf{x}}$ formed for any $\mathbf{x} \in \mathcal{E}$ by the transformations $\pm \boldsymbol{\delta}, \pm \mathbf{g}\left(\pi, \mathbf{n}_{1}\right), \pm \mathbf{g}\left(\pi, \mathbf{n}_{2}\right), \pm \mathbf{g}\left(\pi, \mathbf{n}_{3}\right)$, where $\mathbf{n}_{1}, \mathbf{n}_{2}, \mathbf{n}_{3}$ are three mutually orthogonal unit vectors. A material is called transversally isotropic with respect to the direction $\mathbf{n}$ if its symmetry group $S_{\mathbf{x}}(\forall \mathbf{x} \in \mathcal{E})$ consists of the transformations $\pm \boldsymbol{\delta}$ and $\pm \mathbf{g}(\omega, \mathbf{n})$ with $0<\omega<\pi$.

Depending on the number of rotations and/or reflection symmetry a crystal possesses, the $6 \times 6$ matrix $\mathbf{L}$ can be represented by one of the eight groups summarized in [821]. If the matrix $\mathbf{L}^{\prime}$ referred to a different coordinate system $\mathbf{e}_{i}^{\prime}$ is desired, the transformation (2.160) is used.

Let us consider polycrystalline aggregates of an orthorhombic system. Then for the classes of symmetry with nine independent elastic constants, the tensors of the elastic moduli and compliance of a crystallite in the crystallographic system of coordinates can be represented in the form (see, e.g., [995]):

$$
\begin{align*}
L_{i j k l} & =\sum_{n}\left[\lambda_{n} \delta_{i n} \delta_{j n} \delta_{k n} \delta_{l n}+\mu_{n}\left(\delta_{i n} \delta_{j n} \delta_{k l}+\delta_{i j} \delta_{k n} \delta_{l n}\right)\right. \\
& \left.+\nu_{n}\left(\delta_{i n} \delta_{j k} \delta_{l n}+\delta_{j n} \delta_{i k} \delta_{l n}+\delta_{i n} \delta_{j l} \delta_{k n}+\delta_{j n} \delta_{i l} \delta_{k n}\right)\right]  \tag{2.170}\\
M_{i j k l} & =\sum_{n}\left[p_{n} \delta_{i n} \delta_{j n} \delta_{k n} \delta_{l n}+q_{n}\left(\delta_{i n} \delta_{j n} \delta_{k l}+\delta_{i j} \delta_{k n} \delta_{l n}\right)\right.
\end{align*}
$$

$$
\begin{equation*}
\left.+r_{n}\left(\delta_{i n} \delta_{j k} \delta_{l n}+\delta_{j n} \delta_{i k} \delta_{l n}+\delta_{i n} \delta_{j l} \delta_{k n}+\delta_{j n} \delta_{i l} \delta_{k n}\right)\right] \tag{2.171}
\end{equation*}
$$

where the elastic constants $\lambda_{n}, \mu_{n}, \nu_{n}$ and $p_{n}, q_{n}, r_{n}(n=1,2,3)$ are defined by the matrix representation of the elastic moduli $L_{m n}$ and stiffness $M_{i j}$ in the crystallographic system:

$$
\begin{align*}
\lambda_{1} & =L_{11}+L_{23}+2 L_{44}-\left(L_{12}+L_{13}+2 L_{55}+2 L_{66}\right), \\
\lambda_{2} & =L_{22}+L_{13}+2 L_{55}-\left(L_{12}+L_{23}+2 L_{44}+2 L_{66}\right), \\
\lambda_{3} & =L_{33}+L_{12}+2 L_{66}-\left(L_{13}+L_{23}+2 L_{44}+2 L_{55}\right), \\
2 \mu_{1} & =L_{12}+L_{13}-L_{23}, \quad 2 \nu_{1}=L_{55}+L_{66}-L_{44}, \\
2 \mu_{2} & =L_{12}+L_{23}-L_{13}, \quad 2 \nu_{2}=L_{44}+L_{66}-L_{55}, \\
2 \mu_{3} & =L_{13}+L_{23}-L_{12}, \quad 2 \nu_{3}=L_{44}+L_{55}-L_{66}, \\
p_{1} & =M_{11}+M_{23}+M_{44} / 2-\left(2 M_{12}+2 M_{13}+M_{55}+M_{66}\right) / 2, \\
p_{2} & =M_{22}+M_{13}+M_{55} / 2-\left(2 M_{12}+2 M_{23}+M_{44}+M_{66}\right) / 2, \\
p_{3} & =M_{33}+M_{12}+M_{66} / 2-\left(2 M_{13}+2 M_{23}+M_{44}+M_{55}\right) / 2, \\
2 q_{1} & =M_{12}+M_{13}-M_{23}, \quad 8 r_{1}=M_{55}+M_{66}-M_{44}, \\
2 q_{2} & =M_{12}+M_{23}-M_{13}, \quad 8 r_{2}=M_{44}+M_{66}-M_{55}, \\
2 q_{3} & =M_{13}+M_{23}-M_{12}, \quad 8 r_{3}=M_{44}+M_{55}-M_{66}, \tag{2.172}
\end{align*}
$$

In particular, for tetragonal crystals, the relations $L_{11}=L_{22}, L_{13}=$ $L_{23}, L_{44}=L_{55}$ reduce the number of unknown elastic constants from nine to six:

$$
\begin{align*}
\lambda_{1} & =\lambda_{2}=L_{11}-\left(L_{12}+L_{66}\right), \\
\lambda_{3} & =L_{33}+L_{12}+2 L_{66}-2\left(L_{13}+2 L_{44}\right), \\
2 \mu_{1} & =2 \mu_{2}=L_{12}, \quad 2 \mu_{3}=2 L_{13}-L_{12}, \\
2 \nu_{1} & =2 \nu_{2}=L_{66}, \quad 2 \nu_{3}=2 L_{44}-L_{66} . \tag{2.173}
\end{align*}
$$

For hexagonal crystals, the addition condition $2 L_{66}=L_{11}-L_{12}$ takes place substitution of which into Eq. (2.173) leads to

$$
\begin{align*}
\lambda_{1} & =\lambda_{2}=0, \quad \lambda_{3}=L_{11}+L_{33}-2\left(L_{13}+2 L_{44}\right) \\
2 \mu_{1} & =2 \mu_{2}=L_{12}, \quad 2 \mu_{3}=2 L_{13}-L_{12} \\
4 \nu_{1} & =4 \nu_{2}=L_{11}-L_{12}, \quad 4 \nu_{3}=4 L_{44}+L_{12}-L_{11} \tag{2.174}
\end{align*}
$$

For cubic symmetry, substitution of the conditions $L_{22}=L_{33}, L_{12}=$ $L_{13}, L_{44}=L_{66}$ into Eq. (2.173) gives

$$
\begin{align*}
& \lambda_{1}=\lambda_{2}=\lambda_{3}=L_{11}-L_{12}-2 L_{44} \\
& \mu_{1}=\mu_{2}=\mu_{3}=L_{12} / 2, \quad \nu_{1}=\nu_{2}=\nu_{3}=L_{44} / 2 \tag{2.175}
\end{align*}
$$

At last, a material is called elastically isotropic if its elastic properties are independent of direction. In the case of cubic crystal symmetry this is realized when $L_{44}=\left(L_{11}-L_{12}\right) / 2$. The deviation of the cubic lattice from an isotropic one can be quantified by the so-called Zener anisotropy ratio

$$
\begin{equation*}
Z=\frac{2 L_{44}}{L_{11}-L_{12}} \tag{2.176}
\end{equation*}
$$

Substitution of an additional condition $2 L_{44}=L_{11}-L_{12}$ for the isotropic medium into Eq. (2.175) yields

$$
\begin{align*}
& \lambda_{1}=\lambda_{2}=\lambda_{3}=0, \quad \mu_{1}=\mu_{2}=\mu_{3}=L_{12} / 2 \\
& \nu_{1}=\nu_{2}=\nu_{3}=\left(L_{11}-L_{12}\right) / 2 \tag{2.177}
\end{align*}
$$

The representation of elastic moduli in the reference coordinate system $\left(O x^{\prime}, O y^{\prime}, O z^{\prime}\right)$ can be obtained by the use of rotation $\mathbf{g} \in O^{3}$ from the tensors $\mathbf{L}(2.170)$ and $\mathbf{M}$ (2.171) according to Eqs. (2.160) and (2.161), respectively. The final representation is simplified in exploiting of the identities $g_{i j} \delta_{j n}=g_{i n}$ and $g_{i j} g_{k j}=g_{i k}$ :

$$
\begin{align*}
L_{i j k l}^{\prime} & =\sum_{n}\left[\lambda_{n} g_{i n} g_{j n} g_{k n} g_{l n}+\mu_{n}\left(g_{i n} g_{j n} \delta_{k l}+\delta_{i j} g_{k n} g_{l n}\right)\right. \\
& \left.+\nu_{n}\left(g_{i n} \delta_{j k} g_{l n}+g_{j n} \delta_{i k} g_{l n}+g_{i n} \delta_{j l} g_{k n}+g_{j n} \delta_{i l} g_{k n}\right)\right]  \tag{2.178}\\
M_{i j k l}^{\prime} & =\sum_{n}\left[p_{n} g_{i n} g_{j n} g_{k n} g_{l n}+q_{n}\left(g_{i n} g_{j n} \delta_{k l}+\delta_{i j} g_{k n} g_{l n}\right)\right. \\
& \left.+r_{n}\left(g_{i n} \delta_{j k} g_{l n}+g_{j n} \delta_{i k} g_{l n}+g_{i n} \delta_{j l} g_{k n}+g_{j n} \delta_{i l} g_{k n}\right)\right] . \tag{2.179}
\end{align*}
$$

where the elastic constants $\lambda_{n}, \mu_{n}, \nu_{n}$ and $p_{n}, q_{n}, r_{n}(n=1,2,3)$ are defined by the matrix representations $L_{i j}$ and $M_{i j}$ in the crystallographic coordinate system (2.172).

In particularly, for the tetragonal crystals (2.173),

$$
\begin{align*}
L_{i j k l}^{\prime} & =L_{12} \delta_{i j} \delta_{k l}+L_{66}\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right)+\left(L_{11}-L_{12}-2 L_{66}\right) \sum_{i j k l} \\
& +\left(L_{33}-L_{11}+2 L_{12}-2 L_{13}+4 L_{66}-4 L_{44}\right) T_{i j k l} \\
& +2\left(L_{44}-L_{66}\right)\left(T_{(i k} \delta_{j l)}+T_{(i l} \delta_{j k)}\right)+2\left(L_{13}-L_{12}\right) T_{(i j} \delta_{k l)} \tag{2.180}
\end{align*}
$$

for hexagonal crystals (2.174),

$$
\begin{align*}
L_{i j k l}^{\prime} & =L_{12} \delta_{i j} \delta_{k l}+\left(L_{11}-L_{12}\right)\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right) \\
& +\left(L_{33}+L_{11}-2 L_{13}-4 L_{44}\right) T_{i j k l}+2\left(L_{13}-L_{12}\right) T_{(i j} \delta_{k l)} \\
& +\left(L_{12}-L_{11}+2 L_{44}\right)\left(T_{(i k} \delta_{j l)}+T_{(i l} \delta_{j k)}\right) \tag{2.181}
\end{align*}
$$

for cubic crystals (2.175),

$$
\begin{equation*}
L_{i j k l}^{\prime}=L_{12} \delta_{i j} \delta_{k l}+L_{44}\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right)+\left(L_{11}-L_{12}-2 L_{44}\right) \sum_{i j k l} \tag{2.182}
\end{equation*}
$$

and for the isotropic material (2.177),

$$
\begin{equation*}
L_{i j k l}^{\prime}=L_{12} \delta_{i j} \delta_{k l}+L_{44}\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right) \tag{2.183}
\end{equation*}
$$

Here one introduces the notations

$$
\begin{equation*}
\sum_{i j k l}=\sum_{n=1}^{3} g_{i n} g_{j n} g_{k n} g_{l n}, T_{i j}=g_{i 3} g_{j 3}, T_{i j k l}=g_{i 3} g_{j 3} g_{k 3} g_{l 3} \tag{2.184}
\end{equation*}
$$

where the tensors $T_{i j k l}$ and $\sum_{i j k l}$ show the properties

$$
\begin{equation*}
T_{i i k l}=T_{k l}, T_{i j k l} T_{k l p q}=T_{i j p q}, \quad \sum i i k l=\delta_{k l}, \quad \sum i j k l \quad \sum k l p q=\sum_{i j p q} \tag{2.185}
\end{equation*}
$$

The representation (2.182) for the cubic crystals can be also presented in more detailed form:

$$
\begin{align*}
L_{11}^{\prime} & =L_{11}-2 L\left(g_{11}^{2} g_{12}^{2}+g_{12}^{2} g_{13}^{2}+g_{13}^{2} g_{11}^{2}\right) \\
L_{12}^{\prime} & =L_{12}+L\left(g_{11}^{2} g_{21}^{2}+g_{12}^{2} g_{22}^{2}+g_{13}^{2} g_{23}^{2}\right) \\
L_{44}^{\prime} & =L_{44}+L\left(g_{12}^{2} g_{13}^{2}+g_{22}^{2} g_{32}^{2}+g_{23}^{2} g_{33}^{2}\right), L \equiv L_{11}-L_{12}-2 L_{44} \tag{2.186}
\end{align*}
$$

The inverse transformations from the coefficients $L_{\gamma \nu}$ to $M_{\gamma \nu}$ can be carried out by direct inversion of the matrix of known coefficients because $L_{\gamma \delta} M_{\delta \nu}=\delta_{\gamma \nu}$. However, in some cases the simple relations between the coefficients $L_{\gamma \nu}$ and $M_{\gamma \nu}$ are useful. They are presented below for some symmetry groups described, at most, by seven independent parameters:
Trigonal materials:

$$
\begin{align*}
M_{11}+M_{12} & =L_{33} / l, \quad M_{11}-M_{12}=L_{44} / l^{\prime}, \quad M_{13}=-L_{13} / l \\
M_{14} & =-L_{14} / l^{\prime}, \quad M_{33}=\left(L_{11}+L_{12}\right) / l, \quad M_{44}=\left(L_{11}-L_{12}\right) / l^{\prime} \\
l & \equiv L_{33}\left(L_{11}+L_{12}\right)-2 L_{13}^{2}, \quad l^{\prime} \equiv L_{44}\left(L_{11}-L_{12}\right)-2 L_{14}^{2} . \tag{2.187}
\end{align*}
$$

Hexagonal materials:

$$
\begin{align*}
M_{11}+M_{12} & =L_{33} / l, M_{11}-M_{12}=\left(L_{11}-L_{12}\right)^{-1}, M_{13}=-L_{13} / l \\
M_{33} & =\left(L_{11}+L_{12}\right) / l, M_{44}=L_{44}^{-1} \\
l & \equiv L_{33}\left(L_{11}+L_{12}\right)-2 L_{13}^{2} \tag{2.188}
\end{align*}
$$

Cubic materials:

$$
\begin{align*}
& M_{11}=\left(L_{11}+L_{12}\right)\left(L_{11}-L_{12}\right)^{-1}\left(L_{11}+2 L_{12}\right)^{-1} \\
& M_{12}=-L_{12}\left(L_{11}-L_{12}\right)^{-1}\left(L_{11}+2 L_{12}\right)^{-1} \\
& M_{44}=L_{44}^{-1} \tag{2.189}
\end{align*}
$$

In particular, the elastic constants in the isotropic matrix $\left(\mathbf{A}=\mathbf{L}, \mathbf{M} ; A_{i j}=\right.$ $\left.L_{i j}, M_{i j}\right)$ :

$$
\mathbf{A}=\left(\begin{array}{llllll}
A_{11} & A_{12} & A_{12} & 0 & 0 & 0  \tag{2.190}\\
A_{12} & A_{11} & A_{12} & 0 & 0 & 0 \\
A_{12} & A_{12} & A_{11} & 0 & 0 & 0 \\
0 & 0 & 0 & A_{44} & 0 & 0 \\
0 & 0 & 0 & 0 & A_{44} & 0 \\
0 & 0 & 0 & 0 & 0 & A_{44}
\end{array}\right)
$$

(where $\left.L_{44}=\left(L_{11}-L_{12}\right) / 2, M_{44}=2\left(M_{11}-M_{12}\right)\right)$ are usually written in the engineering notations:

$$
\begin{align*}
L_{11} & =\frac{E}{1+\nu} \frac{1-\nu}{1-2 \nu}, \quad L_{12}=\frac{E}{1+\nu} \frac{\nu}{1-2 \nu}, \quad L_{44}=\frac{E}{2(1+\nu)}  \tag{2.191}\\
M_{11} & =\frac{1}{E}, \quad M_{12}=-\frac{\nu}{E}, \quad M_{44}=\frac{2(1+\nu)}{E} \tag{2.192}
\end{align*}
$$

where two independent constants Young's modulus $E$ and Poisson's ratio $\nu$ can be also used in the tensor representations $\left(\mathbf{N}_{1}=\boldsymbol{\delta} \otimes \boldsymbol{\delta} / 3\right)$

$$
\begin{equation*}
\mathbf{L}=\frac{3 \nu E}{(1+\nu)(1-2 \nu)} \mathbf{N}_{1}+\frac{E}{1+\nu} \mathbf{I}, \quad \mathbf{M}=-\frac{3 \nu}{E} \mathbf{N}_{1}+\frac{1+\nu}{E} \mathbf{I} . \tag{2.193}
\end{equation*}
$$

Another popular representations of the elastic isotropic properties using Lame's constants $\lambda$ and $\mu$ can be formed in both the matrix

$$
\begin{equation*}
L_{11}=\lambda+\mu, L_{12}=\lambda, L_{66}=\mu \tag{2.194}
\end{equation*}
$$

and tensor

$$
\begin{equation*}
\mathbf{L}=3 \lambda \mathbf{N}_{1}+2 \mu \mathbf{I}, \quad \mathbf{M}=3 s \mathbf{N}_{1}+2 q \mathbf{I}, \tag{2.195}
\end{equation*}
$$

notations where the compliances $s$ and $q$ are expressed in terms of elastic constants:

$$
\begin{equation*}
s=\frac{1}{3(3 \lambda+2 \mu)}, \quad q=\frac{1}{4 \mu} . \tag{2.196}
\end{equation*}
$$

Due to the decomposition of the unit tensor $\mathbf{I}$ into the orthogonal bulk $\mathbf{N}_{1}$ and deviatoric $\mathbf{N}_{2}$ tensors (2.16), the isotropic tensors $\mathbf{L}$ and $\mathbf{M}$ can also be decomposed:

$$
\begin{equation*}
\mathbf{L}=(3 k, 2 \mu) \equiv 3 k \mathbf{N}_{1}+2 \mu \mathbf{N}_{2}, \quad \mathbf{M}=(3 p, 2 q) \tag{2.197}
\end{equation*}
$$

where $k$ and $\mu$, called the bulk and shear moduli, are related to the appropriate compliance coefficients $p, q$ by means of the "multiplication table" for the products between the elementary idempotent tensors $\mathbf{N}_{1}$ and $\mathbf{N}_{2}$ (2.17). Then the inverse tensor $\mathbf{L}_{1}^{-1} \equiv \mathbf{M}$, and the product $\mathbf{L}_{1}: \mathbf{L}_{2}$ of such tensors, $\mathbf{L}_{1}=\left(3 k_{1}, 2 \mu_{1}\right)$ and $\mathbf{L}_{2}=\left(3 k_{2}, 2 \mu_{2}\right)$, can be estimated by the compact formulae:

$$
\begin{equation*}
\mathbf{L}_{1}^{-1}=(3 p, 2 q)=\left(\frac{1}{3 k}, \frac{1}{2 \mu}\right), \quad \mathbf{L}_{1}: \mathbf{L}_{2}=\left(9 k_{1} k_{2}, 4 \mu_{1} \mu_{2}\right) \tag{2.198}
\end{equation*}
$$

Table 2.1 shows the relationships between the various elastic constants of isotropic materials.
Table 2.1. Relationships between elastic moduli

| Constant | $\lambda, \mu$ | $K, \mu$ | $\mu, \nu$ | $E, \nu$ | $E, \mu$ |
| :--- | :--- | :--- | :--- | :--- | :--- |
| $\lambda$ | $\lambda$ | $K-\frac{2}{3} \mu$ | $\frac{2 \mu \nu}{1-2 \nu}$ | $\frac{\nu E}{(1+\nu)(1-3 \nu)}$ | $\frac{\mu(E-2 \mu)}{3 \mu-E}$ |
| $\mu$ | $\mu$ | $\mu$ | $\mu$ | $\frac{E}{2(1+\nu)}$ | $\mu$ |
| $K$ | $\lambda+\frac{2}{3} \mu$ | $K$ | $\frac{2 \mu(1-\nu)}{3(1-2 \nu)}$ | $\frac{E}{3(1-2 \nu)}$ | $\frac{E \mu}{3(3 \mu-E)}$ |
| $E$ | $\frac{(3 \lambda+2 \mu) \mu}{\lambda+\mu}$ | $\frac{9 K \mu}{3 K+\mu}$ | $2(1+\nu) \mu$ | $E$ | $E$ |
| $\nu$ | $\frac{\lambda}{2(\lambda+\mu)}$ | $\frac{3 K-2 \mu}{6 K+2 \mu}$ | $\nu$ | $\nu$ | $\frac{E}{2 \mu}-1$ |

The bulk and shear components of the isotropic tensors $\mathbf{L}$ and $\mathbf{M}$ are found through a suitable contraction of the indices of $\mathbf{L}$ and $\mathbf{M}$ in the following relations

$$
\begin{align*}
& 3 k=\frac{1}{3} L_{i i k k}, \quad 2 \mu=\frac{1}{5}\left(L_{i k i k}-\frac{1}{3} L_{i i k k}\right),  \tag{2.199}\\
& 3 p=\frac{1}{3} M_{i i k k}, \quad 2 q=\frac{1}{5}\left(M_{i k i k}-\frac{1}{3} M_{i i k k}\right), \tag{2.200}
\end{align*}
$$

which can be used at the estimation of average moduli of isotropic polycrystals.
Transversely isotropic materials have five independent elastic parameters. If the plane of isotropy coincides with the $x_{1}, x_{2}$-plane, the elastic and compliance matrices $\left(\mathbf{A}=\mathbf{L}, \mathbf{M} ; A_{i j}=L_{i j}, M_{i j}\right)$ :

$$
\mathbf{A}=\left(\begin{array}{llllll}
A_{11} & A_{12} & A_{13} & 0 & 0 & 0  \tag{2.201}\\
A_{12} & A_{11} & A_{13} & 0 & 0 & 0 \\
A_{13} & A_{13} & A_{11} & 0 & 0 & 0 \\
0 & 0 & 0 & A_{44} & 0 & 0 \\
0 & 0 & 0 & 0 & A_{44} & 0 \\
0 & 0 & 0 & 0 & 0 & A_{55}
\end{array}\right)
$$

(where $\left.L_{55}=\left(L_{11}-L_{12}\right) / 2, M_{55}=2\left(M_{11}-M_{12}\right)\right)$ can be presented in engineering notations:

$$
\begin{align*}
& L_{11}=l\left(\frac{1}{E E_{3}}-\frac{\nu_{3}^{2}}{E_{3}^{2}}\right), \quad L_{12}=l\left(\frac{\nu}{E E_{3}}+\frac{\nu_{3}^{2}}{E_{3}^{2}}\right), \quad L_{13}=l \frac{(1+\nu) \nu_{3}}{E E_{3}} \\
& L_{33}=l \frac{1-\nu^{2}}{E^{2}}, \quad L_{44}=\mu_{3}, \quad l=\frac{E^{2} E_{3}^{2}}{(1+\nu)\left[(1-\nu) E_{3}-2 \nu_{3}^{2} E\right]}  \tag{2.202}\\
& M_{11}=\frac{1}{E}, \quad M_{12}=-\frac{\nu}{E}, \quad M_{13}=-\frac{\nu_{3}}{E_{3}}, \quad M_{33}=\frac{1}{E_{3}}, \quad M_{44}=\frac{1}{\mu_{3}} . \tag{2.203}
\end{align*}
$$

Here the Young modulus and the Poisson ratios in the $x_{1}, x_{2}$-plane are the same, say, $E_{1}=E_{2}=E$ and $\nu_{12}=\nu_{21}=\nu$, respectively. The corresponding shear modulus $\mu_{12}=\mu_{21}=\mu$ is formed as $\mu=E / 2(1+\nu)$. Young modulus, Poisson ratio, and the shear modulus associated with the $x_{3}$-direction and a direction in the $x_{1} x_{2}$-plane are denoted by, say, $E_{3}, \nu_{13}=\nu_{23}=\nu_{3}$, and $\mu_{13}=\mu_{23}=\mu_{3}$, respectively.

For the orthotropic case, the elastic and compliance matrices are defined by nine independent elastic parameters and can be presented in a coordinate system coincident with the material symmetry directions as $\left(\mathbf{A}=\mathbf{L}, \mathbf{M} ; A_{i j}=\right.$ $\left.L_{i j}, M_{i j}\right)$ :

$$
\mathbf{A}=\left(\begin{array}{llllll}
A_{11} & A_{12} & A_{13} & 0 & 0 & 0  \tag{2.204}\\
A_{12} & A_{11} & A_{23} & 0 & 0 & 0 \\
A_{13} & A_{23} & A_{11} & 0 & 0 & 0 \\
0 & 0 & 0 & A_{44} & 0 & 0 \\
0 & 0 & 0 & 0 & A_{55} & 0 \\
0 & 0 & 0 & 0 & 0 & A_{66}
\end{array}\right)
$$

where, for example, the compliance matrix in engineering terminology becomes

$$
\mathbf{M}=\left(\begin{array}{cccccc}
1 / E_{1}-\nu_{21} / E_{2}-\nu_{31} / E_{3} & 0 & 0 & 0  \tag{2.205}\\
-\nu_{12} / E_{1} & 1 / E_{2} & -\nu_{32} / E_{3} & 0 & 0 & 0 \\
-\nu_{13} / E_{1}-\nu_{23} / E_{2} & 1 / E_{3} & 0 & 0 & 0 \\
0 & 0 & 0 & 1 / \mu_{23} & 0 & 0 \\
0 & 0 & 0 & 0 & 1 / \mu_{13} & 0 \\
0 & 0 & 0 & 0 & 0 & 1 / \mu_{12}
\end{array}\right)
$$

We will now consider a unique representation of the constitutive laws and elastic moduli for a $d$-dimensional linear isotropic material with bulk modulus $k_{[d]}$, shear modulus $\mu_{[d]}$, Young's modulus $E_{[d]}$, and Poisson's ratio $\nu_{[d]}$ in the space dimensionality $d(i, j, k, l=1, \ldots, d ; d=2,3)$ (see, e.g., [1106]):

$$
\begin{equation*}
\sigma_{i j}=\left(k_{[d]}-\frac{2}{d} \mu_{[d]}\right) \varepsilon_{k k} \delta_{i j}+2 \mu_{[d]} \varepsilon_{i j}, \quad \varepsilon_{i j}=-\frac{\nu_{[d]}}{E_{[d]}} \sigma_{k k} \delta_{i j}+\frac{\left(1+\nu_{[d]}\right)}{E_{[d]}} \sigma_{i j} \tag{2.206}
\end{equation*}
$$

and

$$
\begin{equation*}
\mathbf{L}=d k_{[d]} \mathbf{N}_{1}+2 \mu_{[d]} \mathbf{N}_{2}, \quad \mathbf{M}=\frac{1+\nu_{[d]}(1-d)}{E_{[d]}} \mathbf{N}_{1}+\frac{1+\nu_{[d]}}{E_{[d]}} \mathbf{N}_{2} \tag{2.207}
\end{equation*}
$$

where the projection tensors

$$
\begin{equation*}
\mathbf{N}_{1 \mid i j k l}=\frac{1}{d} \delta_{i j} \delta_{k l}, \quad \mathbf{N}_{2 \mid i j k l}=\frac{1}{2}\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right)-\frac{1}{d} \delta_{i j} \delta_{k l} \tag{2.208}
\end{equation*}
$$

satisfy the normality conditions (2.17) allowing the inverse of relation (2.207 ${ }_{1}$ )

$$
\begin{equation*}
\mathbf{M}=\frac{1}{d k_{[d]}} \mathbf{N}_{1}+\frac{1}{2 \mu_{[d]}} \mathbf{N}_{2} \tag{2.209}
\end{equation*}
$$

Comparison of representations $\left(2.207_{2}\right)$ and (2.209) leads to the interrelations

$$
\begin{align*}
k_{[d]} & =\frac{E_{[d]}}{d\left[1+\nu_{[d]}(1-d)\right]}, \quad \mu_{[d]}=\frac{E_{[d]}}{2\left(1+\nu_{[d]}\right)} \\
\frac{1}{E_{[d]}} & =\frac{1}{d^{2} k_{[d]}}+\frac{d-1}{2 d \mu_{[d]}}, \quad \nu_{[d]}=\frac{d k_{[d]}-2 \mu_{[d]}}{d(d-1) k_{[d]}+2 \mu_{[d]}} \tag{2.210}
\end{align*}
$$

Passages to the limits of the positive moduli $k_{[d]} / \mu_{[d]} \rightarrow \infty$ and $\mu_{[d]} / k_{[d]} \rightarrow \infty$ in the expression $\left(2.210_{4}\right)$ yield the upper and lower limits of $\nu_{[d]}:-1 \leq \nu_{[d]} \leq$ $(d-1)^{-1}$, respectively.

Finally, we will consider either plane-strain ( $\varepsilon_{11}=\varepsilon_{12}=\varepsilon_{13}$ in Eq. (2.206) with $d=3$ ) or plane-stress ( $\sigma_{11}=\sigma_{12}=\sigma_{13}$ in Eq. (2.206) with $d=3$ ) elasticity. If we compare these simplified three-dimensional expressions to relations (2.206) with $d=2$, we obtained the interrelations:

$$
\begin{equation*}
E_{[2]}=\frac{E_{[3]}}{\left(1+\nu_{[3]}\right)\left(1+\nu_{[3]}\right)}, \quad \nu_{[2]}=\frac{\nu_{[3]}}{1-\nu_{[3]}}, \quad k_{[2]}=k_{[3]}+\frac{1}{3} \mu_{[3]}, \quad \mu_{[2]}=\mu_{[3]}, \tag{2.211}
\end{equation*}
$$

for plane-strain elasticity and

$$
\begin{equation*}
E_{[2]}=E_{[3]}, \quad \nu_{[2]}=\nu_{[3]}, \quad k_{[2]}=\frac{9 k_{[3]} \mu_{[3]}}{3 k_{[3]}+4 \mu_{[3]}}, \quad \mu_{[2]}=\mu_{[3]} \tag{2.212}
\end{equation*}
$$

for plane-stress elasticity, respectively.

### 2.6 Basic Equations of Thermoelastoplastic Deformations

### 2.6.1 Incremental Theory of Plasticity

We assume that the thermomechanical properties of the composite medium with, generally speaking, anisotropic components are described by the theory of small elastic-plastic strains under arbitrarily varying external loading. Additive decomposition of the increments of total strain tensor $\varepsilon$ is assumed:

$$
\begin{equation*}
d \boldsymbol{\varepsilon}=d \varepsilon^{e}+d \varepsilon^{t}+d \varepsilon^{p} \tag{2.213}
\end{equation*}
$$

with the increments of the elastic strains $d \varepsilon^{e}$, the so-called "transformation strains" $d \varepsilon^{t}$ including thermal strains and plastic strains $d \varepsilon^{p}$.

The stress increment $d \boldsymbol{\sigma}$ relates to the elastic part of the strain increment $d \boldsymbol{\varepsilon}$ by the elasticity relation given in the form

$$
\begin{equation*}
d \boldsymbol{\sigma}(\mathbf{x})=\mathbf{L}(\mathbf{x}) d \boldsymbol{\varepsilon}^{e}(\mathbf{x}) \tag{2.214}
\end{equation*}
$$

where $\mathbf{L}(\mathbf{x})$ is the fourth-order elasticity tensor (2.88).
The transformation strain increment $d \boldsymbol{\varepsilon}^{t}(2.213)$ may consist of contributions of different physical origins. For example, if only thermal effects are considered, $\boldsymbol{\varepsilon}^{t}=\boldsymbol{\beta}^{T} \theta$, where $\boldsymbol{\beta}^{T}$ is the tensor of linear thermal expansion coefficients and $\theta$ is the temperature change from the reference value to the current temperature.

In the six-dimensional stress space, consider a yield surface $f(\mathbf{x}, \ldots)=0$ bonding the region of plastic deformation, in which $f(\boldsymbol{\sigma}, \ldots)>0$; here dots stand for temperature and for internal variables characterizing material hardening. The behavior is elastic if $f<0$, or if $f=0$ and $[(\partial f / \partial \boldsymbol{\sigma}): d \boldsymbol{\sigma}+(\partial f / \partial \theta) d \theta] \leq 0$ (for the elastic unloading and neutral loading); elastic-plastic deformations take place under active loading, when $f=0$ and $[(\partial f / \partial \boldsymbol{\sigma}): d \boldsymbol{\sigma}+(\partial f / \partial \theta) d \theta]>0$. In the general case, the yield surface can depend on a variety of tensor $\mathbf{a}^{p}$ and scalar hardening parameters. The hardening tensor parameter, in particular, may simply coincide with the plastic-strain tensor. Regarding the constitutive equations for the elastic-plastic materials we use the so-called $J_{2}$-flow theory with combined isotropic-kinematic hardening. The von Mises form of the yield surface is given by

$$
\begin{equation*}
f \equiv \tau-F(\gamma, \theta)=0, \quad F(0, \theta)=\tau_{0}(\theta) \tag{2.215}
\end{equation*}
$$

in terms of von Mises effective stress, $\tau$, and the accumulated effective plastic strain increment, $\gamma$, respectively, as defined by

$$
\begin{equation*}
\tau=\left(\frac{3}{2} s_{i j}^{a} s_{i j}^{a}\right)^{1 / 2}, \quad d \gamma=\left(\frac{2}{3} d \varepsilon_{i j}^{p} d \varepsilon_{i j}^{p}\right)^{1 / 2}, \quad \mathbf{s}^{a}=\mathbf{N}_{2}\left(\boldsymbol{\sigma}-\mathbf{a}^{p}\right) \tag{2.216}
\end{equation*}
$$

Here $\tau_{0}$ is the initial yield stress, $F$ is a nonlinear function describing the hardening effect and temperature dependence, for example as in the modified Ludwik equation:

$$
\begin{equation*}
F(\gamma, \theta)=\tau_{0}(\theta)+h(\theta) \gamma^{n(\theta)} \tag{2.217}
\end{equation*}
$$

where $h$ and $n$ are hardening parameters. $\mathbf{s}^{a}$ is the active stress deviator; $\mathbf{a}^{p}$ is a symmetric second order tensor corresponding to the "back-stresses" defining the location of the center of the yield surface in the deviatoric stress space. For evaluation of the back stress tensor $\mathbf{a}^{p}$ we use Ziegler's assumption:

$$
\begin{equation*}
d \mathbf{a}^{p}=d \gamma A \mathbf{s}^{a}, \quad A \equiv A(\gamma) \tag{2.218}
\end{equation*}
$$

or Prager's rule

$$
\begin{equation*}
d \mathbf{a}^{p}=B d \varepsilon^{p}, \quad B \equiv B(\gamma) \tag{2.219}
\end{equation*}
$$

respectively, with $\mathbf{a}^{p}=0$ if $\gamma=0$. In the case $\mathbf{a}^{p} \equiv \mathbf{0}, h \neq 0$, Eqs. (2.215) and (2.216) are reduced to the isotropic hardening; the case $h \equiv 0, \mathbf{a}^{p} \neq \mathbf{0}$ correspondents to the kinematic hardening. Though the von Mises yield criterion (2.215) is assumed in this study, modifications of the present method for introducing other yield criteria and hardening laws are possible.

Drucker's postulate states that the work of additional stresses is positive over the whole cycle. Then for any stress state $\sigma^{0}$ within or on the convex yield surface the local maximum principle $\left(\boldsymbol{\sigma}-\boldsymbol{\sigma}^{0}\right): d \boldsymbol{\varepsilon}^{p} \geq 0$. Drucker's postulate leads to the necessity of the associated law of plastic flow when the smooth yield function $f$ is taken as plastic potential function from which the incremental plastic strain can be derived as

$$
\begin{equation*}
\varepsilon^{p}=d \lambda \frac{\partial f}{\partial \boldsymbol{\sigma}}, \quad \text { for } \quad f=0, \quad \frac{\partial f}{\partial \boldsymbol{\sigma}}: d \boldsymbol{\sigma}>0 \tag{2.220}
\end{equation*}
$$

where the plastic flow is fixed in the direction along the normal to the yield surface, while its magnitude $d \lambda$, called a proportionality factor, is undetermined. For consistency during plastic deformation:

$$
\begin{equation*}
d f \equiv \frac{\partial f}{\partial \boldsymbol{\sigma}}: d \boldsymbol{\sigma}+\frac{\partial f}{\partial \mathbf{a}^{p}}: d \mathbf{a}^{p}+\frac{\partial f}{\partial \gamma} d \gamma+\frac{\partial f}{\partial \theta} d \theta=0 \tag{2.221}
\end{equation*}
$$

Substitution of $(2.216),(2.218),(2.219)$ in (2.221) leads to the determination of $d \lambda$ by

$$
\begin{equation*}
d \lambda=G\left(\frac{\partial f}{\partial \boldsymbol{\sigma}}: d \boldsymbol{\sigma}+\frac{\partial f}{\partial \theta} d \theta\right) \tag{2.222}
\end{equation*}
$$

where the proportionality parameter $G$ is defined by the explicit form of hardening rule (2.217)-(2.219). For example, for isotropic-kinematic hardening, (2.217), (2.218), we have

$$
\begin{equation*}
G=-\left(\frac{\partial f}{\partial \gamma}+A(\gamma) \frac{\partial f}{\partial \mathbf{a}^{p}}: \mathbf{s}^{a}\right)^{-1}\left(\frac{2}{3} \frac{\partial f}{\partial \boldsymbol{\sigma}}: \frac{\partial f}{\partial \boldsymbol{\sigma}}\right)^{-1 / 2} \tag{2.223}
\end{equation*}
$$

The associated flow law (2.220) requires the smoothness of the yield surface with a defined normal to the surface. However, the plastic flow corresponding to edges or vertexes of the yield surface $f=0$ originated by the intersections of smooth surfaces $f_{\alpha}=0(\alpha=1, \ldots)$ can be estimated from superposition principle by Koiter [581]:

$$
\begin{align*}
d \varepsilon^{p} & =\sum_{\alpha=1} d \lambda_{\alpha} \frac{\partial f_{\alpha}}{\partial \boldsymbol{\sigma}}  \tag{2.224}\\
f_{\alpha} & =0, \quad \frac{\partial f_{\alpha}}{\partial \boldsymbol{\sigma}}: d \boldsymbol{\sigma} \geq 0 \tag{2.225}
\end{align*}
$$

A state of stress on the yield surface is described by a value of zero of one or more yield functions, all other yield functions being negative. In so doing, the nonzero proportionality factors $d \lambda_{\alpha}$ correspond only to the yield surfaces satisfying the conditions (2.225).

### 2.6.2 Deformation Theory of Plasticity

It is assumed that the rheological properties of isotropic media are described by the theory of small elastoplastic strains under monotonic, proportional loading when the ratio of the stress components $\sigma_{11}: \sigma_{22}: \sigma_{33}: \sigma_{13}: \sigma_{23}: \sigma_{12}$ is held constant at all time. Specifically, the total strain $\varepsilon$ is written as the sum of elastic $\varepsilon^{e}$ and plastic $\varepsilon^{p}$ contributions:

$$
\begin{equation*}
\varepsilon=\varepsilon^{e}+\varepsilon^{p} \tag{2.226}
\end{equation*}
$$

where the mean stress and mean strain are linearly related: $\sigma_{i i}=3 k \varepsilon_{i i}$. The local equation for the elastic material state, which relates the stress tensor $\boldsymbol{\sigma}(\mathbf{x})$ and the elastic strain tensor $\boldsymbol{\varepsilon}^{e}(\mathbf{x})$, is given in the form (2.214) where $\mathbf{L}(\mathbf{x})$ is an isotropic fourth-order elasticity tensor described by the Young's modulus $E$ and the Poisson's ratio $\nu$ (2.193).

The relation between flow stress and plastic strains is represented by the expression

$$
\begin{equation*}
\tau=\tau_{0}+f\left(\varepsilon_{\mathrm{eq}}^{p}\right), \quad f(0)=0 \tag{2.227}
\end{equation*}
$$

in terms of the von Mises effective stress $\tau$ and the effective plastic strain $\epsilon_{\text {eq }}^{p}$, defined as

$$
\begin{equation*}
\tau=\left(\frac{3}{2} s_{i j} s_{i j}\right)^{1 / 2}, \quad \varepsilon_{\mathrm{eq}}^{p}=\left(\frac{2}{3} \varepsilon_{i j}^{p} \varepsilon_{i j}^{p}\right)^{1 / 2}, \quad \mathbf{s} \equiv \mathbf{N}_{2} \sigma \tag{2.228}
\end{equation*}
$$

In Eq. (2.227), $\tau_{0}$ is the initial yield stress and $f$ is a nonlinear function describing the material's hardening behavior, for example:

$$
\begin{equation*}
f\left(\epsilon_{\mathrm{eq}}^{p}\right)=h\left(\epsilon_{\mathrm{eq}}^{p}\right)^{n} \tag{2.229}
\end{equation*}
$$

where $h$ and $n$ are the strength coefficient and the work-hardening exponent, respectively. The material remains elastic, i.e., $\boldsymbol{\varepsilon}^{p} \equiv \mathbf{0}$, when $\tau<\tau_{0}$. In addition, Hencky's flow rule

$$
\begin{equation*}
\varepsilon_{i j}^{p}=\frac{3 \varepsilon_{\mathrm{eq}}^{p}}{2 \tau} \sigma_{i j} \tag{2.230}
\end{equation*}
$$

is adopted. A more general relation given in Prager's form is

$$
\begin{equation*}
\varepsilon_{i j}^{p}=\alpha s_{i j}+\beta t_{i j} \tag{2.231}
\end{equation*}
$$

where $t_{i j}=s_{i k} s_{k j}-2 / 3 J_{2} \delta_{i j}$ is the deviation of the square of the stress deviation, and for an isotropic materials $\alpha$ and $\beta$ are the functions of the invariants $J_{2}=$ $s_{i j} s_{i j} / 2$ and $J_{3}=s_{i j} s_{j k} s_{k i} / 3$.

Based on relation (2.226) one may determine the secant modulus as

$$
\begin{equation*}
E^{s}=\left[\frac{1}{E}+\frac{\epsilon_{\mathrm{eq}}^{p}}{\tau_{0}+f\left(\epsilon_{\mathrm{eq}}^{p}\right)}\right]^{-1} \tag{2.232}
\end{equation*}
$$

Due to the plastic incompressibility the secant bulk modulus $k^{s}$ is equal to $k$, and we may accordingly define the secant Poisson's ratio and shear modulus:

$$
\begin{equation*}
\nu^{s}=\frac{1}{2}-\left(\frac{1}{2}-\nu\right) \frac{E^{s}}{E}, \quad \mu^{s}=\frac{E^{s}}{2\left(1+\nu^{s}\right)} \tag{2.233}
\end{equation*}
$$

Both here and below the superscript ${ }^{s}$ indicates the calculation of the parameter under consideration with the help of the secant modulus $E^{s}$. Therefore, under a monotonic, proportional loading, the plastic state can be described by a single secant modulus, say $E^{s}$, and the other two elastic constants. For these loading the incremental flow rule can be integrated to yield the deformation rule.

## Conclusion. Critical Analysis of Some Basic Concepts of Micromechanics

> T is strange,- but true, for Truth is always strangeStranger than fiction ...

-Lord Byron. Don Juan.
If you can look into the seeds of time,
And say which grain will grow and which will not, Speak then to me ...
-William Shakespeare. Macbeth.

Let us discuss the main scheme as well as the short sketch of limitations and ideas as well as possible generalizations of some basic concepts of the linear version of the MEFM and some related methods (see Chapters 7-14). This sketch does not pretend to be rigorous and may contain controversial statements, too personal or one-sided arguments which are deliberately presented with the aim to give free reins to our imagination.

Let us consider the basic equations of linear elasticity of composites in micropoint (see Chapter 8 for details):

$$
\begin{equation*}
\nabla \boldsymbol{\sigma}=\mathbf{0}, \quad \boldsymbol{\sigma}(\mathbf{x})=\mathbf{L}(\mathbf{x}) \varepsilon(\mathbf{x}), \quad \varepsilon(\mathbf{x})=\operatorname{Def} \mathbf{u}(\mathbf{x}) \tag{19.1}
\end{equation*}
$$

Substituting (19.12) and (19.13) into the equilibrium equation (19.1 $)$ we obtain a differential equation:

$$
\begin{equation*}
\nabla \mathbf{L}^{c} \nabla \mathbf{u}(\mathbf{x})=-\nabla \mathbf{L}_{1}(\mathbf{x}) \nabla \mathbf{u}(\mathbf{x}) \tag{19.2}
\end{equation*}
$$

with the constant coefficients in the left-hand side that allows the general integral equation (7.19)

$$
\begin{equation*}
\varepsilon(\mathbf{x})=\langle\varepsilon\rangle(\mathbf{x})+\int \mathbf{U}^{c}(\mathbf{x}-\mathbf{y})[\boldsymbol{\tau}(\mathbf{y})-\langle\boldsymbol{\tau}\rangle] d \mathbf{y} . \tag{19.3}
\end{equation*}
$$

where the tensor $\mathbf{U}^{c}=\nabla \nabla \mathbf{G}^{c}$ is defined by the Green function $\mathbf{G}^{c}$ (3.3) for the infinite homogeneous medium with an elastic modulus $\mathbf{L}^{c}$.

Equation (19.3) can be rearranged in the form of an infinite system of integral equations (8.8)-(8.11) that can be solved by the different approximate methods
considered in Chapters 8 and 9. The main hypothesis of many micromechanical methods called effective field hypothesis $\mathbf{H 1}$ ) (8.14) and (8.15) allows the infinite system for integral equations with respect the smooth effective fields for any shape and structure of inclusions [see Eqs. (8.18) and (8.19)]. However, a generality of the last statements is an illusion because arbitrariness of the inclusion shape does not guarantee a protection from the necessity of using of Eshelby tensor in the MEFM. Indeed, the notion Eshelby tensor has a fundamental conceptual sense rather than only an analytical solution of some particular problem. Exploiting the Eshelby tensor concept in the MEFM is based on the ellipsoidal shape of the correlation hole $v_{i j}^{0}$ rather than on the inclusion shape $v_{i}$. An abandonment of the assumption of the $v_{i j}^{0}$ 's ellipsoidal shape leads of necessity to the inhomogeneity of the effective field $\overline{\boldsymbol{\sigma}}_{i}$ acting on the inclusion $v_{i}$ that is prohibited for the classical version of the MEFM (see Sections 9.4).

For statistically homogeneous medium subjected to homogeneous boundary conditions, two-particle approximation of a closing hypothesis H2 (8.39):

$$
\begin{equation*}
\left\langle\widetilde{\boldsymbol{\sigma}}(\mathbf{x})_{1,2}\right\rangle_{i}=\langle\overline{\boldsymbol{\sigma}}(\mathbf{x})\rangle_{i}=\text { const. } \quad(i=1,2) \tag{19.4}
\end{equation*}
$$

reduces this system to the system of algebraic Eqs. (8.40). Further simplification is accomplished by "quasi-crystalline" approximation by Lax (8.65):

$$
\begin{equation*}
\left\langle\overline{\boldsymbol{\sigma}}_{i}(\mathbf{x}) \mid v_{i}, \mathbf{x}_{i} ; v_{j}, \mathbf{x}_{j}\right\rangle=\left\langle\overline{\boldsymbol{\sigma}}_{i}\right\rangle, \quad \mathbf{x} \in v_{i} \tag{19.5}
\end{equation*}
$$

which is weaker than Mori-Tanaka assumption (8.90):

$$
\begin{equation*}
\overline{\boldsymbol{\sigma}}_{i} \equiv\langle\boldsymbol{\sigma}\rangle^{(0)} \tag{19.6}
\end{equation*}
$$

The Mori-Tanaka hypothesis (19.6) is probably the most popular hypothesis of micromechanics which at the same time exerted most deleterious impact on the development of this subject. Harmful effect is explained not only by the loss of the effective modulus symmetry (see Subsections 8.3.3 and Section 18.3) in some general cases but, that is crucial, by self-closing nature of the hypothesis (19.6). The seeming simplicity and illusory assuredness hide the ways of generalization of this hypothesis while an extension of "quasi-crystalline" approximation (19.5) to the closing hypothesis (19.4) is obvious.

Summarizing what we said above, we will present now the contractions of some concepts and assumptions erroneously recognized in micromechanics as basic ones: a) constitutive equation, b) homogeneous comparison medium, c) Green function, d) Eshelby tensor, e) effective field hypothesis H1.

The fallacy in these recognitions will be justified in an inverse order.
Indeed, we demonstrated in the book two methods of analyses of the inhomogeneous effective fields $\bar{\sigma}_{i}$. In the first case of statistically homogeneous medium subjected to the homogeneous boundary conditions, the inhomogeneity of $\bar{\sigma}_{i}$ takes place if the correlation hole $v_{i j}^{0}$ is nonellipsoidal that leads to the problem of the estimation of stress distribution in the domain $v_{i j}^{0}$ undergoing to the constant eigenstrains $\boldsymbol{\beta}_{1}(\mathbf{x}) \equiv$ const, $\mathbf{L}_{1}(\mathbf{x}) \equiv \mathbf{0}\left(\mathbf{x} \in v_{i j}^{0}\right)$ (see Subsections 4.2.3 and Section 9.4). A more general second method applicable for the analysis of both statistically homogeneous and statistically inhomogeneous fields
in both the infinite and bounded media, and for any shape of $v_{i j}^{0}$ is reduced to the estimation of perturbations produced by the inclusion $v_{j}$ in both inside and outside in some vicinity of the inclusion $v_{j}$ (see Eq. (14.63)). In so doing these perturbations evaluated by any either numerical or analytical method available for a researcher are reduced to the classical Eshelby internal and external tensors only in some particular simplest cases.

Thus the popular effective field hypothesis H1 is not fundamental and can be easily avoided if it is known the numerical solution just for one inclusion obtained by any numerical method available for researcher. Known numerical methods such as FEA, VIE, BIE, and the complex potentials, which can be used for micromechanical analysis, have a series of advantages and disadvantages. It is crucial for the analyst to be aware of their range of applications. This solution only in simplest cases is expressed through the Green function and Eshelby tensor which are not, in general, necessary tools in micromechanics. In the case of taking of multiparticle interactions into account, a solution for a single inclusion should be complemented by a solution for $n$ interacting inclusions in the comparison medium that also can be performed without the notions Green function and Eshelby tensor (see Chapters 10 and 14) by the use of operator representations of these solutions. In so doing, the existence of the mentioned operators was justified through the Green function's technique.

The challenge of modern micromechanics is a development of the general methodology incorporating the solution for multiple interacting inhomogeneities obtained by highly accurate numerical methods into the most general scheme of analytical micromechanics (see Chapters 10-12, and 14). A fundamental difference between the proposed methodology and the ones published earlier is a systematic analysis of statistical distributions $\left\langle\boldsymbol{\sigma}(\otimes \boldsymbol{\sigma})^{n}\right\rangle^{(k)}(\mathbf{x})(n=0,1, \ldots ; k=$ $\left.0,1, \ldots, N ; \mathbf{x} \in v^{(k)}\right)$ of local microfields rather than only effective properties based on the average fields inside the phases $\langle\boldsymbol{\sigma}\rangle^{(k)}$. Moreover, this approach allows us to estimate the conditional moments of stresses inside the fixed inclusions $v_{i}:\left\langle\boldsymbol{\sigma}(\otimes \boldsymbol{\sigma})^{n}\right\rangle_{i}\left(\mathbf{x}^{l}\right)\left(\mathbf{x}^{l} \in v_{i} \subset v^{(k)}\right)$, which are more informative than $\left\langle\boldsymbol{\sigma}(\otimes \boldsymbol{\sigma})^{n}\right\rangle^{(k)}(\mathbf{x})\left(\mathbf{x} \in v^{(k)}\right)$ (see Chapter 14). Obviously, the analysis of nonlinear phenomena such as plasticity, creep, and damage should use precisely $\left\langle\boldsymbol{\sigma}(\otimes \boldsymbol{\sigma})^{n}\right\rangle_{i}\left(\mathbf{x}^{l}\right)\left(\mathbf{x}^{l} \in v_{i} \subset v^{(k)}\right)$, rather than $\left\langle\boldsymbol{\sigma}(\otimes \boldsymbol{\sigma})^{n}\right\rangle^{(k)}(\mathbf{x})\left(\mathbf{x} \in v^{(k)}\right)$ (see Chapters 15 and 16), which is a too rough descriptor of the stress distribution especially in the case of statistically inhomogeneous media in bounded domains (see Chapters 10, 13, and 14).

Furthermore, popular assumptions about homogeneity of the comparison medium and their unboundedness (19.2) were in reality accepted exclusively for introduction of a much more powerful tool of micromechanical research such as Green's function; see Eq. (19.3). However, the eventual abandonment of the Green function concept (see Chapters 10, 11, and 14) removed any restrictions on the comparison medium in the case of linear elastic problems. A comparison medium can either coincide with the whole space or be bounded by any surface (either simple connected or multiply connected). Elastic properties and eigenstrains of the comparison medium can be either homogeneous or inhomogeneous. A single restriction on the inhomogeneity of a comparison medium is a
deterministic nature of this inhomogeneity. So, a comparison medium can have either continuously varying elastic moduli or piecewise constant elastic properties such as e.g. periodic structure (see Chapter 11) or two joint elastic half spaces. The bounded or unbounded medium with one or a few fixed macroinclusions (e.g. macrocracks) can also be considered as a comparison medium. However, at any rate, we need only to know the numerical solutions for the mentioned comparison medium containing one and a few interacting micro inclusions with any their possible location in the comparison medium.

At last, must we really know a local constitutive law (19.12) if we are going to estimate the effective elastic moduli of composite materials? At first glance this question is beyond the scope of common sense. However, despite the apparent paradoxicality, the correct answer to this question is no, in the following sense. Indeed, it was demonstrated in Section 18.2 that we don't need to know anything about either local or nonlocal nature of constitutive laws of inclusions. We only need to know that these laws are linear and also need to know the concentrator factors $\mathbf{A}_{\sigma}(\mathbf{x})$ and $\mathbf{B}_{\sigma}(\mathbf{x})$ (18.10) (which can be found directly, e.g., from MD simulations) at least for one inclusion in a comparison medium mentioned above.

Shortening what we said above, we can conclude that for linear elastic problems for microinhomogeneous medium of random structure:
a) we do not need to know a constitutive law for the inclusions,
b) we do not need to assume the homogeneity and infiniteness of a comparison medium,
c) we do not need to know the Green tensor and Eshelby tensor, and
d) we do not need to assume homogeneity of the effective fields $\overline{\boldsymbol{\sigma}}(\mathbf{x})$.

What we really need is the numerical solutions for one and a few interacting inclusions with any their possible location in the comparison medium described above.

Thus, in the framework of a unique scheme of the proposed MEFM, we have undertaken in this book an attempt to analyze the wide class of statical and dynamical, local and nonlocal, linear and nonlinear multiscale problems of composite materials with deterministic (periodic and nonperiodic), random (statistically homogeneous and inhomogeneous, so-called graded) and mixed (periodic structures with random imperfections) structures in bounded and unbounded domains, containing coated or uncoated inclusions of any shape and orientation and subjected to coupled or uncoupled, homogeneous or inhomogeneous external fields of different physical natures.

